

Stormwater Management Report

for:

Common Driveway

85 & 95 West Street
Northborough, MA 01532

Project Proponent:

Brant L. Viner
Margaret Harling

P.O. Box 295
Ellsworth, ME 04605

REVISED
May 3, 2021

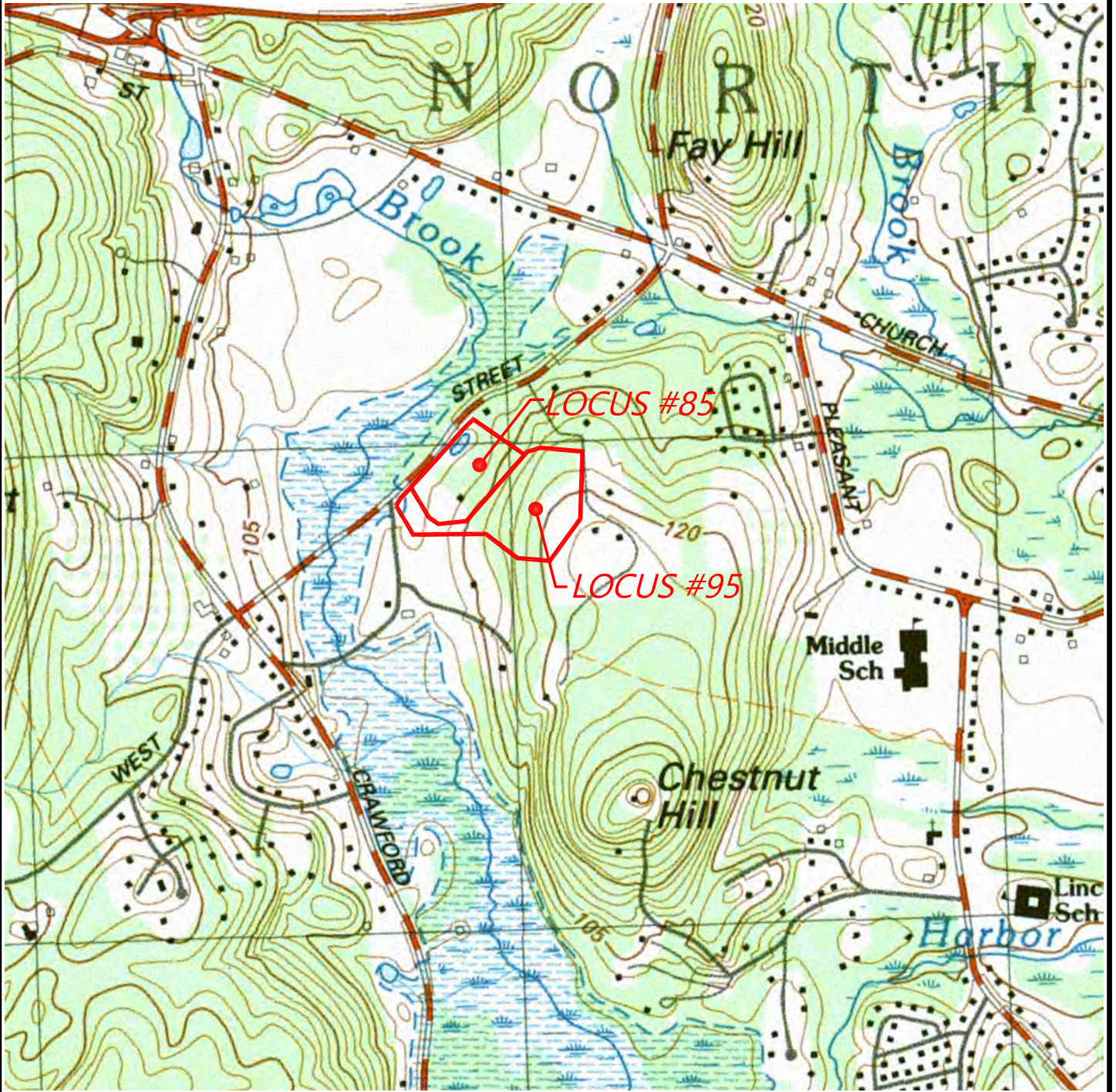



Gregory B. Scotchie, P.E.



TABLE OF CONTENTS	TOC
Locus Map.....	1
Purpose.....	2
Project Site	2
Proposed Project.....	4
Stormwater Management Standards	4
Standard #1 – No New Untreated Discharges	4
Standard #2 – Peak Rate Attenuation	4
Methodology.....	4
Analysis Summary.....	5
Standard #3 – Stormwater Recharge	7
Standard #4 – Water Quality volume	7
Standard #5 – Land Uses with Higher Potential Pollutant Loads (LUHPPLs).....	8
Standard #6 – Critical Areas.....	8
Standard #7 – Redevelopment Project.....	8
Standard #8 – Construction Pollution Prevention and Erosion and Sedimentation Control	8
Standard #9 – Operation and Maintenance Plan.....	8
Standard #10 – Prohibition of Illicit Discharges.....	8
Massachusetts Stormwater Report Checklist (Attached)	9
Existing Hydrology.....	9
Proposed Hydrology	24
Appendices	86
Massachusetts Stormwater Report Checklist	
Operation & Maintenance and Long-Term Pollution Prevention Plan	
Stormwater Standards Calculations	
Hydraulics	
NOAA Atlas 14 Point Precipitation Frequency Estimates	
Maps:	
Soils Map Area of Detail and Soil Testing Field Data	
FEMA / NFIP / FIRM	
Existing Hydrology Plan	
Proposed Hydrology Plan	

LOCUS MAP



PREPARED BY:



31 EAST MAIN STREET WESTBOROUGH, MA | 508.366.6552
 7 CENTRAL STREET PROVIDENCE, RI | 401.274.1360
 WDA-DG.COM

TITLE: USGS LOCUS PLAN
 85 & 95 West Street
 Northborough, MA

PREPARED FOR: BRANT L. VINER &
 MARGARET HARLING
 PO Box 295
 Ellsworth, ME 04605

SOURCE: USGS Topographic Map

DATE:	07/10/18	FILE NO.:	1207001	DRAWN BY:	BPW
JOB NO.:	1207.00	DWG NO.:		REV DATE:	—/—/—
				REV. NO.:	—



PURPOSE

Hydrologic, hydraulic, and water quality analysis and design calculations have been performed in accordance with the Town of Northborough Stormwater Management Rules and Regulations for the site work proposed on a common driveway located at 85 & 95 West Street in Northborough, MA. The calculations were performed to design stormwater collection and attenuation facilities for the site and to demonstrate that the project will meet the stormwater management standards of the Town of Northborough and the Massachusetts Department of Environmental Protection (MassDEP) Stormwater Management Regulations.

This report describes the existing project site, the proposed project, and analyses performed to develop a stormwater management system that will protect public safety and convenience and minimize environmental impacts.

PROJECT SITE

The Locus Parcels are located on the south side of West Street. 85 West Street contains a single-family home and outbuildings, with a paved driveway accessing from West Street and contains approximately 6.6 acres and has 623.34' of frontage along West Street, beginning approximately 235 northeast of the Cherry Street intersection. 95 West Street is an undeveloped parcel, generally located behind (east) of 85 West Street, but also with frontage along West Street. The undeveloped land at 95 West Street contains 10.7 ± acres and has 235.54 feet of frontage along West Street with the frontage beginning at the intersection with Cherry Street. The properties are located entirely within the Residential B (RB) Zoning District, with a portion of the front area of the lots contained within the Zone 3 Groundwater Overlay District. The property is bound to north by West Street and land of the Commonwealth (DCR) to the north side of the road, and by single family development around the east, south and west. A portion of each property contains an area of Zone A, 100-year flood zone, as shown on the Flood Insurance Rate Map for the Town of Northborough (Community Panel # 25027C0633F, effective date July 16, 2014). The flood zone is associated with Cold Harbor Brook which is located several hundred feet to the north of the property. The flood zone noted for 85 West Street encompasses a small pond located at the front of the property adjacent to West Street. This pond discharges flows via a riser pipe to a culvert under West Street which discharges flows into an intermittent stream channel to the north. This flood zone is indicated at elevation 321.3 to 321.4'. The flood zone noted for 95 West Street is shown located within the field area near the corner of West Street and Cherry Street. This also is connected to a culvert which conveys flows from the field area under Cherry Street. This flood zone in this location is noted as elevation 318.7 to 318.8'. The Property does not contain a designated area of priority habitat of rare species or wildlife, or vernal pools (potential or certified), according to the Natural Heritage Atlas, valid from August 1, 2017.

Soils mapped on-site by the NRCS are primarily Paxton fine sandy loam (map units 305C, 306B/C and 307D), Windsor loamy sand (255B), Chatfield-Holli-Rock outcrop complex (102C) and Woodbridge fine sandy loam (312B).

Two areas of bordering vegetated wetlands were identified on the property, as well as a small, isolated wetland. The isolated wetland is very small (150 sf) and would only be jurisdictional under the local Wetlands Bylaw. WDA delineated the on-site resource areas on April 27, 2018 and these boundaries were determined upon review of the existing vegetation, soils and hydrologic conditions, as well as other visual indicators. Soils along the wetland boundary were reviewed utilizing a hand auger and Munsell soil color chart. All flags have been field survey located and shown on the attached plans.

The first and largest wetland delineated is located on the land of 85 West Street, generally bordering the small pond and adjacent seeps. This wetland is delineated with blue survey flagging, labeled by flag series WF-1 to WF-91. A small area of upland located in the interior of the wetland was delineated near the easterly side of the wetland, this is defined by flags WF-UA to WF-UI. The wetland boundary begins generally at the edge of the access driveway and West Street and runs along West Street, around the pond and back into the front portion of the property. The pond appears to be a historically manmade pond, approximately 17,000 sf surface area. A 12" vertical standpipe located within the pond, near the West Street side, discharges highwater flows into a 12" corrugated metal culvert which runs under West Street and into an intermittent stream channel. This stream eventually connects with Cold Harbor Brook, approximately 250' to the northwest. Two seeps, one located near the northerly property boundary and the other on the south end of the pond, feed intermittent flows down to the pond, and generally the wetland flagging around and associated with the pond and the seep are delineated by flags WF-20 to WF-74. A natural drainage divide near WF-74 also directs seasonal seeps to the west, near the driveway and West Street, essentially flags WF-19 to WF-74. During our site visit on April 27, standing water was observed generally around WF-1 to WF-5 and WF-78 to WF-82 and WF-87-91. The wetland areas as defined above are predominately wooded shrub with herbaceous understory, specifically: red maple, elm, gray birch, pussy willow, silky dogwood, highbush blueberry, winterberry, cattail, jewelweed, skunk cabbage, cinnamon and sensitive fern. Adjacent upland areas are dominated by white pine, oak, black and white birch, hickory, ash, red maple, sugar maple, black cherry, hornbeam, teaberry, Pennsylvania sedge and multiflora rose.

The second wetland area is located on the 95 West Street property, within a field area in the southwest portion of the property, near the West and Cherry Street intersection. This wetland receives additional seasonal seeps or stormflows from a shallow grass channel which emanates from a culvert under an adjacent driveway and lawn area, located off site to the south. A shallow channel located at the westerly edge of the wetland directs seasonal flows/seeps to a culvert under Cherry Street. This wetland system is predominately a wet meadow, containing herbaceous species and limited shrubs and trees. This wetland system is delineated by flag series WF-100 to WF-119. Dominate species identified during our site visit were; soft rush, reed canary grass, sensitive fern, goldenrod, cattail, goldenrod, pussy willow, silky dogwood and red maple. Adjacent upland field areas, although not fully grown out, had indicators of miscellaneous field grasses, ground ivy, clover, and goldenrod. These areas have been historically mowed as well.

The final wetland area delineated is a small, isolated wet area located along the southern edge of the driveway, approximately 115' southeast of West Street, where the driveway splits into two. This wetland is approximately 150 square feet in size and is covered under the Northborough Wetland bylaw and Regulations. This area is delineated by flag series IW-A to IW-E. Wetland vegetation in this area is comprised mainly of; buttonbush, soft rush, tussock sedge, woolgrass and goldenrod.

The attached Existing Hydrology Plan shows the project design point(s) and contributing drainage areas with existing land cover types. The analyzed design points are (DP-1) near the intersection of Cherry Street and West Street, and the 12" vertical standpipe in the pond (DP-2).

PROPOSED PROJECT

The proposed work is considered a new development project. The applicant is proposing to build a common drive to subdivide the above-mentioned lots into six (6) lots with single-family dwellings constructed per Town of Northborough Development Standards. The project will utilize conventional stormwater management techniques including grass lined swales, deep sump catch basins, subsurface infiltration chambers, and a stormwater pond for the treatment and mitigation of stormwater. Clearing of the site will occur to accommodate the common driveway and associated infrastructure for the residential lots.

A stormwater management plan has been developed to meet the requirements of the Town of Northborough and the MassDEP Stormwater Standards. Runoff will flow overland via sheet flow or shallow concentrated flow towards grass swales along either side of the common drive. Water quality inlets will collect the flow from the grassed swale before the flow enters the subsurface infiltration and dry extended detention systems.

Soils mapped on-site by the NRCS are primarily Paxton fine sandy loam (map units 305C, 306B/C and 307D), Windsor loamy sand (255B), Chatfield-Holli-Rock outcrop complex (102C) and Woodbridge fine sandy loam (312B). Three of the five underground basins are within the Windsor loamy sand (255B) and the other two infiltration basins are in the Paxton fine sandy loam. The three underground basins with Windsor loamy sand are dry extended detention ponds due to poor infiltration rates. The proposed detention pond will not infiltrate stormwater due to the proximity of the 100-yr floodplain.

STORMWATER MANAGEMENT STANDARDS

STANDARD #1 – NO NEW UNTREATED DISCHARGES

The stormwater collection systems have been designed so that stormwater runoff from the asphalt paved areas is treated through a treatment train consisting of grass lined swales, deep-sump, hooded drop inlets, stormwater infiltration system and a detention basin. Additionally, all outlets have been designed so that there will be no erosion or scour to the wetlands of the Commonwealth.

STANDARD #2 – PEAK RATE ATTENUATION

METHODOLOGY

United States Soil Conservation Service, "Urban Hydrology for Small Watersheds, Technical Release Number 55 (TR-55) methods (HydroCAD 10.10-4b) were utilized to develop runoff hydrographs for watershed areas affected by the proposed development. Existing and proposed runoff hydrographs were developed for the 2-, 10-, and 100-year, 24-hour rainfall events to develop a stormwater management system that will limit post-development peak runoff rates to pre-development levels.

The proposed stormwater management plan has been designed to meet the requirements of the Town of Northborough and the MassDEP Stormwater Management Standards. The project will limit peak rates of runoff from the site and will infiltrate runoff to approximate existing groundwater recharge.

ANALYSIS SUMMARY

To assess the impact of the proposed development on peak runoff rates onto down-gradient properties, hydrologic calculations were performed for each of three (3) design storms at the two (2) design points. The calculations refer to runoff quantities at the final design points, Design Point 1 (DP-1) near the intersection of Cherry Street and West Street and the other, DP-2) is an existing pond adjacent to West Street. Stormwater runoff from the rooftops will be routed to infiltration chambers located on their respective lots.

Calculations of peak runoff rates for existing and proposed site conditions are summarized in Table I for comparison of peak runoff rates for the design point for the three design storm events. A proposed hydrology plan is provided showing the various sub-watersheds draining to the proposed stormwater management facilities. Stormwater runoff from the overland areas not tributary to the stormwater management facilities will drain by sheet flow or shallow concentrated flow along the existing flow patterns to the design points.

Table I demonstrates that the proposed stormwater management system will be effective in limiting peak rates of runoff from the subject property to approximate pre-development levels. Table II indicates the change in stormwater runoff volume. Table III and IV provide data on the existing and proposed ground cover areas.

TABLE I: EXISTING AND PROPOSED PEAK RUNOFF

DRAINAGE AREA	DESIGN STORM EVENT / PEAK RUNOFF (cfs)		
	2-Year	10-Year	100-Year
Existing (DP-1)	2.5	7.7	18.0
Proposed (DP-1)	1.3	5.9	18.0
Existing (DP-2)	6.6	16.3	34.1
Proposed (DP-2)	5.3	15.3	33.9

TABLE II: EXISTING AND PROPOSED RUNOFF VOLUMES

DRAINAGE AREA	DESIGN STORM EVENT / VOLUME (cf)		
	2-Year	10-Year	100-Year
Existing (DP-1)	16,513	43,029	95,956
Proposed (DP-1)	6,813	30,953	85,108
Existing (DP-2)	34,323	78,960	162,568
Proposed (DP-2)	29,004	73,050	157,291

TABLE III: EXISTING GROUND COVER

EXISTING GROUND COVER	HYDROLOGIC SOIL GROUP (HSG)				
	HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Total (sq-ft)
>75% Grass cover, Good	65,785	0	49,420	11,365	126,570
Paved Parking	10	0	7,674	2,340	10,025
Roofs	0	0	2,657	0	2,657
Water	0	0	11,813	5,791	17,604
Woods, Good	0	25,377	550,292	21,760	597,430
Total Area	65,795	25,377	621,856	41,257	754,285

TABLE IV: PROPOSED GROUND COVER

PROPOSED GROUND COVER	HYDROLOGIC SOIL GROUP (HSG)				
	HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Total (sq-ft)
>75% Grass cover, Good	57,233	0	168,545	9,399	235,177
Paved Parking	8,556	0	39,995	420	45,971
Roofs	0	0	16,716	0	16,716
Water	0	0	11,813	5,791	17,604
Woods, Good	0	25,377	387,792	25,648	438,817
Total Area	65,789	25,377	621,861	41,258	754,285

TABLE V: WATER SURFACE ELEVATIONS

STORMWATER FACILITY	100-YR STORM EVENT WATER ELEVATION (FT)	TOP/BERM ELEVATION	Existing Ground Elevations (ft)	Proposed Grade Elevations (ft)	Bottom of Stone/Invert (ft)	Redox Elevation (ft)	Provided Separation to SHGW (ft)	Observed Infiltration Rates
POND (1P)	320.7	322.0	321.0	320.0	N/A	N/A	N/A	N/A
INF-103	327.8	N/A	327.0	328.7	326.0	323.0	3.0	0.276
INF-104	328.6	N/A	325.0	329.6	325.1	323.0	2.1	3.072
INF -105	349.9	N/A	347.2	351.0	346.5	344.1	2.4	1.284
INF -106	357.9	N/A	355.0	359.0	351.3	349.2	2.1	1.758
INF -201	339.7	N/A	336.2	340.7	336.2	333.2	3.0	2.262

STANDARD #3 – STORMWATER RECHARGE

Groundwater recharge is provided within the five (5) subsurface infiltration systems INF-103, -104, -105, -106, and -201. The test pits have been performed throughout the site and in areas where infiltration is proposed. The test pit data is provided in the appendix. Soil textures identified in the C horizon were Loam-Fine Sandy Loam for Pond (1P), Loamy Sand for INF-103, Medium Sand for INF-104, Fine Sandy Loam for INF-105 and 106, and Loamy Sand for INF-201. Double-ring infiltrometer tests were performed to determine infiltration rates for INF-103, -104. A hydraulic conductivity by Guelph permeameter test was performed to determine infiltration rates for INF-105, -106, and 201. Infiltration testing data is provided in the Appendix. The proposed detention pond will not infiltrate stormwater due to high ground water level. The Static Method was used in sizing the infiltration systems.

The table below provides a summary of the attached groundwater recharge calculations. Calculations are based on HSG A, C, and D. The required volume of groundwater recharge is equal to 1.0", 0.25" and 0.1" over the proposed impervious area for A, C, and D soils.

REQUIRED (CF)	PROVIDED (CF)
1,648	12,049

CAPTURE AREA ADJUSTMENT

Total Impervious Area = 80,291 sf
 Impervious Area Draining to Infiltration Facilities = 45,378 sf
 Ratio = 80,291 / 45,378 = 1.77
 *Adjusted Required Infiltration Volume (Rv) = 1.77 x 1,648 cf = 2,916 cf

ADJUSTED REQUIRED (CF)	PROVIDED (CF)
2,916	12,049

DRAWDOWN CALCULATIONS

$$Time = \frac{Rv}{(K)(Bottom\ Area)}$$

Rv = Storage Volume (cubic feet)
 K = Saturated Hydraulic Conductivity (inches per hour)

Drawdown times for all basins are shown in the Stormwater Standards Calculations section of the appendix.

STANDARD #4 – WATER QUALITY VOLUME

Water quality measures will be designed to provide a minimum of 80% Total Suspended Solids (TSS) removal, and to treat 0.5 of runoff prior to discharging to the bordering vegetated wetland. The water quality volume

is achieved by providing a static storage volume below the outlet in the infiltration basin and within the subsurface infiltration facility.

REQUIRED (CF)	PROVIDED (CF)
2,325	12,049

Over 80% TSS removal will be provided through the use of the following two treatment trains: 1) grass lined swale 2) deep sump drop inlet, 3) subsurface infiltration systems, or 1) grass lined swale 2) deep sump drop inlet, 3) extended dry detention pond. Water quality calculations for all outfalls are shown in the Stormwater Standards Calculations section of the appendix.

STANDARD #5 – LAND USES WITH HIGHER POTENTIAL POLLUTANT LOADS (LUHPPLS)

The proposed project is not considered a land use with Higher Potential Pollutant Loads therefore, Standard #5 is not applicable.

STANDARD #6 – CRITICAL AREAS

The proposed project will not discharge near or to a Critical Area. Appropriate water quality and TSS removal Best Management Practices (BMPs) have been specified to satisfy Standard #6.

STANDARD #7 – REDEVELOPMENT PROJECT

The proposed project not considered a redevelopment project therefore, Standard #7 is not applicable.

STANDARD #8 – CONSTRUCTION POLLUTION PREVENTION AND EROSION AND SEDIMENTATION CONTROL

As the total project area is over one acre, a Notice of Intent (NOI) must be filed with the US EPA and a Stormwater Pollution Prevention Plan (SWPPP) shall be retained on-site during construction. The attached draft project SWPPP has been developed in accordance with the current MA Construction General Permit (CGP). The draft SWPPP is included as part of this Stormwater Management Report for compliance with the Standards.

STANDARD #9 – OPERATION AND MAINTENANCE PLAN

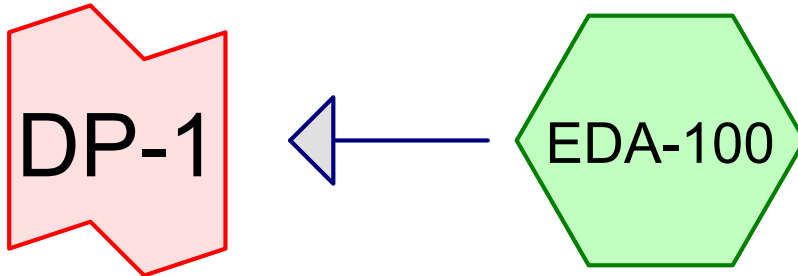
The attached Operation and Maintenance Plan describes the requisite long-term operation and maintenance of all on-site stormwater Best Management Practices (BMPs) and hydraulic drainage system. The Operation and Maintenance Plan also describes source control for the prevention of pollution to also serve as the Long-Term Pollution Prevention Plan (LTPPP).

STANDARD #10 – PROHIBITION OF ILLICIT DISCHARGES

An Illicit Discharge Compliance Statement will be provided by the Owner/Applicant prior to the discharge of stormwater to post-construction BMPs.

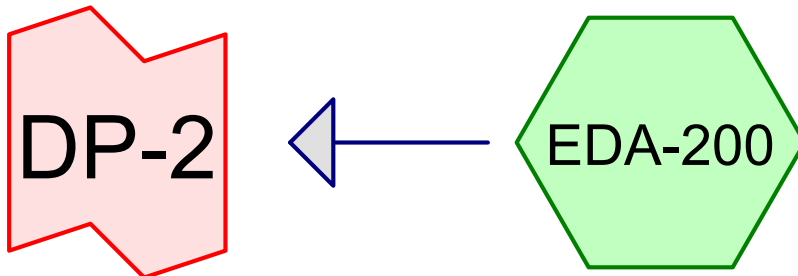
MASSACHUSETTS STORMWATER REPORT CHECKLIST (Attached)

EXISTING HYDROLOGY



DESIGN POINT 1

EDA-100



DESIGN POINT 2

EDA-200

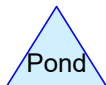
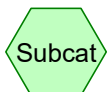


TABLE OF CONTENTS

Project Reports

- 10 Routing Diagram
- 11 Rainfall Events Listing (selected events)
- 12 Area Listing (all nodes)
- 13 Soil Listing (all nodes)
- 14 Ground Covers (all nodes)

2-year Event

- 15 Node Listing
- 16 Subcat EDA-100: EDA-100
- 17 Subcat EDA-200: EDA-200
- 18 Link DP-1: DESIGN POINT 1
- 18 Link DP-2: DESIGN POINT 2

10-year Event

- 18 Node Listing
- 19 Subcat EDA-100: EDA-100
- 20 Subcat EDA-200: EDA-200
- 21 Link DP-1: DESIGN POINT 1
- 21 Link DP-2: DESIGN POINT 2

100-year Event

- 21 Node Listing
- 22 Subcat EDA-100: EDA-100
- 23 Subcat EDA-200: EDA-200
- 24 Link DP-1: DESIGN POINT 1
- 24 Link DP-2: DESIGN POINT 2

85 West Street Northborough JN-1207.03 - Existing 2021-05-03

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 11

Rainfall Events Listing (selected events)

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-year	Type III 24-hr		Default	24.00	1	3.28	2
2	10-year	Type III 24-hr		Default	24.00	1	5.02	2
3	100-year	Type III 24-hr		Default	24.00	1	7.78	2

85 West Street Northborough JN-1207.03 - Existing 2021-05-03

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 12

Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
65,785	39	>75% Grass cover, Good, HSG A (EDA-100)
49,420	74	>75% Grass cover, Good, HSG C (EDA-100, EDA-200)
11,365	80	>75% Grass cover, Good, HSG D (EDA-100, EDA-200)
10	98	Paved parking, HSG A (EDA-100)
7,674	98	Paved parking, HSG C (EDA-100, EDA-200)
2,340	98	Paved parking, HSG D (EDA-100, EDA-200)
2,657	98	Roofs, HSG C (EDA-100, EDA-200)
11,813	98	Water Surface, HSG C (EDA-200)
5,791	98	Water Surface, HSG D (EDA-200)
25,377	55	Woods, Good, HSG B (EDA-200)
550,292	70	Woods, Good, HSG C (EDA-100, EDA-200)
21,760	77	Woods, Good, HSG D (EDA-200)
754,285	69	TOTAL AREA

85 West Street Northborough JN-1207.03 - Existing 2021-05-03

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 13

Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
65,795	HSG A	EDA-100
25,377	HSG B	EDA-200
621,856	HSG C	EDA-100, EDA-200
41,257	HSG D	EDA-100, EDA-200
0	Other	
754,285		TOTAL AREA

85 West Street Northborough JN-1207.03 - Existing 2021-05-03

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 14

Ground Covers (all nodes)

HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover
65,785	0	49,420	11,365	0	126,570	>75% Grass cover, Good
10	0	7,674	2,340	0	10,025	Paved parking
0	0	2,657	0	0	2,657	Roofs
0	0	11,813	5,791	0	17,604	Water Surface
0	25,377	550,292	21,760	0	597,430	Woods, Good
65,795	25,377	621,856	41,257	0	754,285	TOTAL AREA

Time span=0.00-96.00 hrs, dt=0.05 hrs, 1921 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-100: EDA-100 Runoff Area=309,776 sf 3.82% Impervious Runoff Depth=0.64"
Flow Length=1,237' Tc=27.7 min CN=65 Runoff=2.5 cfs 16,513 cf

Subcatchment EDA-200: EDA-200 Runoff Area=444,509 sf 4.15% Impervious Runoff Depth=0.93"
Flow Length=985' Tc=21.7 min CN=71 Runoff=6.6 cfs 34,323 cf

Link DP-1: DESIGN POINT 1 Inflow=2.5 cfs 16,513 cf
Primary=2.5 cfs 16,513 cf

Link DP-2: DESIGN POINT 2 Inflow=6.6 cfs 34,323 cf
Primary=6.6 cfs 34,323 cf

Total Runoff Area = 754,285 sf Runoff Volume = 50,835 cf Average Runoff Depth = 0.81"
95.98% Pervious = 724,000 sf 4.02% Impervious = 30,285 sf

85 West Street Northborough JN-1207.03 - Existing 2021 Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 16

Summary for Subcatchment EDA-100: EDA-100

Runoff = 2.5 cfs @ 12.47 hrs, Volume= 16,513 cf, Depth= 0.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
38,421	74	>75% Grass cover, Good, HSG C
187,813	70	Woods, Good, HSG C
10	98	Paved parking, HSG A
65,785	39	>75% Grass cover, Good, HSG A
2,362	98	Roofs, HSG C
7,657	98	Paved parking, HSG C
1,789	98	Paved parking, HSG D
5,939	80	>75% Grass cover, Good, HSG D
309,776	65	Weighted Average
297,958		96.18% Pervious Area
11,818		3.82% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.2	50	0.0140	0.06		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
7.4	684	0.0940	1.53		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
6.1	503	0.0385	1.37		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
27.7	1,237	Total			

Summary for Subcatchment EDA-200: EDA-200

Runoff = 6.6 cfs @ 12.34 hrs, Volume= 34,323 cf, Depth= 0.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-year Rainfall=3.28"

85 West Street Northborough JN-1207.03 - Existing 2021 Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 17

Area (sf)	CN	Description
25,377	55	Woods, Good, HSG B
362,479	70	Woods, Good, HSG C
10,999	74	>75% Grass cover, Good, HSG C
295	98	Roofs, HSG C
11,813	98	Water Surface, HSG C
17	98	Paved parking, HSG C
5,791	98	Water Surface, HSG D
551	98	Paved parking, HSG D
21,760	77	Woods, Good, HSG D
5,426	80	>75% Grass cover, Good, HSG D
444,509	71	Weighted Average
426,042		95.85% Pervious Area
18,467		4.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.9	50	0.0270	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
10.8	935	0.0826	1.44		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
21.7	985	Total			

Summary for Link DP-1: DESIGN POINT 1

Inflow Area = 309,776 sf, 3.82% Impervious, Inflow Depth = 0.64" for 2-year event
 Inflow = 2.5 cfs @ 12.47 hrs, Volume= 16,513 cf
 Primary = 2.5 cfs @ 12.47 hrs, Volume= 16,513 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: DESIGN POINT 2

Inflow Area = 444,509 sf, 4.15% Impervious, Inflow Depth = 0.93" for 2-year event
 Inflow = 6.6 cfs @ 12.34 hrs, Volume= 34,323 cf
 Primary = 6.6 cfs @ 12.34 hrs, Volume= 34,323 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.05 hrs

Time span=0.00-96.00 hrs, dt=0.05 hrs, 1921 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-100: EDA-100 Runoff Area=309,776 sf 3.82% Impervious Runoff Depth=1.67"
Flow Length=1,237' Tc=27.7 min CN=65 Runoff=7.7 cfs 43,029 cf

Subcatchment EDA-200: EDA-200 Runoff Area=444,509 sf 4.15% Impervious Runoff Depth=2.13"
Flow Length=985' Tc=21.7 min CN=71 Runoff=16.3 cfs 78,960 cf

Link DP-1: DESIGN POINT 1 Inflow=7.7 cfs 43,029 cf
Primary=7.7 cfs 43,029 cf

Link DP-2: DESIGN POINT 2 Inflow=16.3 cfs 78,960 cf
Primary=16.3 cfs 78,960 cf

Total Runoff Area = 754,285 sf Runoff Volume = 121,990 cf Average Runoff Depth = 1.94"
95.98% Pervious = 724,000 sf 4.02% Impervious = 30,285 sf

Summary for Subcatchment EDA-100: EDA-100

Runoff = 7.7 cfs @ 12.42 hrs, Volume= 43,029 cf, Depth= 1.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.05 hrs
 Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
38,421	74	>75% Grass cover, Good, HSG C
187,813	70	Woods, Good, HSG C
10	98	Paved parking, HSG A
65,785	39	>75% Grass cover, Good, HSG A
2,362	98	Roofs, HSG C
7,657	98	Paved parking, HSG C
1,789	98	Paved parking, HSG D
5,939	80	>75% Grass cover, Good, HSG D
309,776	65	Weighted Average
297,958		96.18% Pervious Area
11,818		3.82% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.2	50	0.0140	0.06		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
7.4	684	0.0940	1.53		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
6.1	503	0.0385	1.37		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
27.7	1,237	Total			

Summary for Subcatchment EDA-200: EDA-200

Runoff = 16.3 cfs @ 12.31 hrs, Volume= 78,960 cf, Depth= 2.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.05 hrs
 Type III 24-hr 10-year Rainfall=5.02"

85 West Street Northborough JN-1207.03 - Existing 20 Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 20

Area (sf)	CN	Description
25,377	55	Woods, Good, HSG B
362,479	70	Woods, Good, HSG C
10,999	74	>75% Grass cover, Good, HSG C
295	98	Roofs, HSG C
11,813	98	Water Surface, HSG C
17	98	Paved parking, HSG C
5,791	98	Water Surface, HSG D
551	98	Paved parking, HSG D
21,760	77	Woods, Good, HSG D
5,426	80	>75% Grass cover, Good, HSG D
444,509	71	Weighted Average
426,042		95.85% Pervious Area
18,467		4.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.9	50	0.0270	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
10.8	935	0.0826	1.44		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
21.7	985	Total			

Summary for Link DP-1: DESIGN POINT 1

Inflow Area = 309,776 sf, 3.82% Impervious, Inflow Depth = 1.67" for 10-year event
 Inflow = 7.7 cfs @ 12.42 hrs, Volume= 43,029 cf
 Primary = 7.7 cfs @ 12.42 hrs, Volume= 43,029 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: DESIGN POINT 2

Inflow Area = 444,509 sf, 4.15% Impervious, Inflow Depth = 2.13" for 10-year event
 Inflow = 16.3 cfs @ 12.31 hrs, Volume= 78,960 cf
 Primary = 16.3 cfs @ 12.31 hrs, Volume= 78,960 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.05 hrs

Time span=0.00-96.00 hrs, dt=0.05 hrs, 1921 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-100: EDA-100 Runoff Area=309,776 sf 3.82% Impervious Runoff Depth=3.72"
Flow Length=1,237' Tc=27.7 min CN=65 Runoff=18.0 cfs 95,956 cf

Subcatchment EDA-200: EDA-200 Runoff Area=444,509 sf 4.15% Impervious Runoff Depth=4.39"
Flow Length=985' Tc=21.7 min CN=71 Runoff=34.1 cfs 162,568 cf

Link DP-1: DESIGN POINT 1 Inflow=18.0 cfs 95,956 cf
Primary=18.0 cfs 95,956 cf

Link DP-2: DESIGN POINT 2 Inflow=34.1 cfs 162,568 cf
Primary=34.1 cfs 162,568 cf

Total Runoff Area = 754,285 sf Runoff Volume = 258,524 cf Average Runoff Depth = 4.11"
95.98% Pervious = 724,000 sf 4.02% Impervious = 30,285 sf

85 West Street Northborough JN-1207.03 - Existing 2Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 22

Summary for Subcatchment EDA-100: EDA-100

Runoff = 18.0 cfs @ 12.40 hrs, Volume= 95,956 cf, Depth= 3.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
38,421	74	>75% Grass cover, Good, HSG C
187,813	70	Woods, Good, HSG C
10	98	Paved parking, HSG A
65,785	39	>75% Grass cover, Good, HSG A
2,362	98	Roofs, HSG C
7,657	98	Paved parking, HSG C
1,789	98	Paved parking, HSG D
5,939	80	>75% Grass cover, Good, HSG D
309,776	65	Weighted Average
297,958		96.18% Pervious Area
11,818		3.82% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.2	50	0.0140	0.06		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
7.4	684	0.0940	1.53		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
6.1	503	0.0385	1.37		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
27.7	1,237	Total			

Summary for Subcatchment EDA-200: EDA-200

Runoff = 34.1 cfs @ 12.30 hrs, Volume= 162,568 cf, Depth= 4.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-year Rainfall=7.78"

85 West Street Northborough JN-1207.03 - Existing 2Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 23

Area (sf)	CN	Description
25,377	55	Woods, Good, HSG B
362,479	70	Woods, Good, HSG C
10,999	74	>75% Grass cover, Good, HSG C
295	98	Roofs, HSG C
11,813	98	Water Surface, HSG C
17	98	Paved parking, HSG C
5,791	98	Water Surface, HSG D
551	98	Paved parking, HSG D
21,760	77	Woods, Good, HSG D
5,426	80	>75% Grass cover, Good, HSG D
444,509	71	Weighted Average
426,042		95.85% Pervious Area
18,467		4.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.9	50	0.0270	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
10.8	935	0.0826	1.44		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
21.7	985	Total			

Summary for Link DP-1: DESIGN POINT 1

Inflow Area = 309,776 sf, 3.82% Impervious, Inflow Depth = 3.72" for 100-year event
 Inflow = 18.0 cfs @ 12.40 hrs, Volume= 95,956 cf
 Primary = 18.0 cfs @ 12.40 hrs, Volume= 95,956 cf, Atten= 0%, Lag= 0.0 min

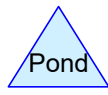
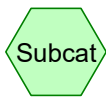
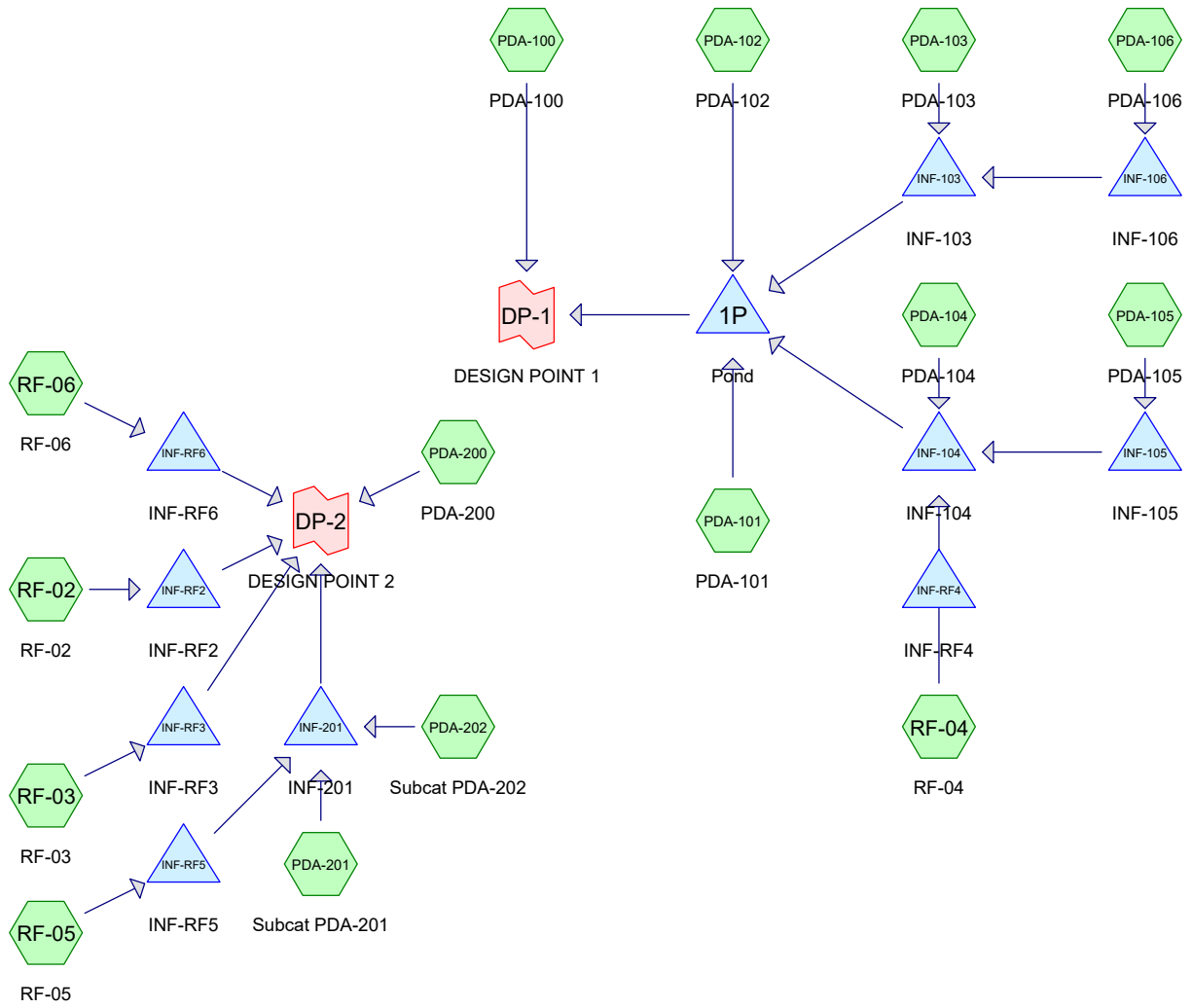
Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.05 hrs

Summary for Link DP-2: DESIGN POINT 2

Inflow Area = 444,509 sf, 4.15% Impervious, Inflow Depth = 4.39" for 100-year event
 Inflow = 34.1 cfs @ 12.30 hrs, Volume= 162,568 cf
 Primary = 34.1 cfs @ 12.30 hrs, Volume= 162,568 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.05 hrs

PROPOSED HYDROLOGY



Routing Diagram for 85 West Street Northborough JN-1207.03 - Proposed

Prepared by WDA Design Group, Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

85 West Street Northborough JN-1207.03 - Proposed

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

TABLE OF CONTENTS**Project Reports**

- 25 Routing Diagram
- 26 Rainfall Events Listing (selected events)
- 27 Area Listing (all nodes)
- 28 Soil Listing (all nodes)
- 29 Ground Covers (all nodes)

2-year Event

- 30 Node Listing
- 32 Subcat PDA-100: PDA-100
- 33 Subcat PDA-101: PDA-101
- 33 Subcat PDA-102: PDA-102
- 34 Subcat PDA-103: PDA-103
- 35 Subcat PDA-104: PDA-104
- 35 Subcat PDA-105: PDA-105
- 36 Subcat PDA-106: PDA-106
- 36 Subcat PDA-200: PDA-200
- 37 Subcat PDA-201: Subcat PDA-201
- 38 Subcat PDA-202: Subcat PDA-202
- 38 Subcat RF-02: RF-02
- 39 Subcat RF-03: RF-03
- 39 Subcat RF-04: RF-04
- 39 Subcat RF-05: RF-05
- 39 Subcat RF-06: RF-06
- 40 Pond 1P: Pond
- 40 Pond INF-103: INF-103
- 41 Pond INF-104: INF-104
- 42 Pond INF-105: INF-105
- 43 Pond INF-106: INF-106
- 44 Pond INF-201: INF-201
- 44 Pond INF-RF2: INF-RF2
- 45 Pond INF-RF3: INF-RF3
- 46 Pond INF-RF4: INF-RF4
- 47 Pond INF-RF5: INF-RF5
- 47 Pond INF-RF6: INF-RF6
- 48 Link DP-1: DESIGN POINT 1
- 48 Link DP-2: DESIGN POINT 2

10-year Event

- 49 Node Listing
- 51 Subcat PDA-100: PDA-100
- 52 Subcat PDA-101: PDA-101
- 52 Subcat PDA-102: PDA-102
- 53 Subcat PDA-103: PDA-103
- 54 Subcat PDA-104: PDA-104

85 West Street Northborough JN-1207.03 - Proposed

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

54	Subcat PDA-105: PDA-105
55	Subcat PDA-106: PDA-106
55	Subcat PDA-200: PDA-200
56	Subcat PDA-201: Subcat PDA-201
57	Subcat PDA-202: Subcat PDA-202
57	Subcat RF-02: RF-02
58	Subcat RF-03: RF-03
58	Subcat RF-04: RF-04
58	Subcat RF-05: RF-05
58	Subcat RF-06: RF-06
59	Pond 1P: Pond
59	Pond INF-103: INF-103
60	Pond INF-104: INF-104
61	Pond INF-105: INF-105
62	Pond INF-106: INF-106
63	Pond INF-201: INF-201
63	Pond INF-RF2: INF-RF2
64	Pond INF-RF3: INF-RF3
65	Pond INF-RF4: INF-RF4
66	Pond INF-RF5: INF-RF5
66	Pond INF-RF6: INF-RF6
67	Link DP-1: DESIGN POINT 1
67	Link DP-2: DESIGN POINT 2

100-year Event

68	Node Listing
70	Subcat PDA-100: PDA-100
71	Subcat PDA-101: PDA-101
71	Subcat PDA-102: PDA-102
72	Subcat PDA-103: PDA-103
73	Subcat PDA-104: PDA-104
73	Subcat PDA-105: PDA-105
74	Subcat PDA-106: PDA-106
74	Subcat PDA-200: PDA-200
75	Subcat PDA-201: Subcat PDA-201
76	Subcat PDA-202: Subcat PDA-202
76	Subcat RF-02: RF-02
77	Subcat RF-03: RF-03
77	Subcat RF-04: RF-04
77	Subcat RF-05: RF-05
77	Subcat RF-06: RF-06
78	Pond 1P: Pond
78	Pond INF-103: INF-103
79	Pond INF-104: INF-104
80	Pond INF-105: INF-105
81	Pond INF-106: INF-106
82	Pond INF-201: INF-201

85 West Street Northborough JN-1207.03 - Proposed

Prepared by WDA Design Group

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

- 82 Pond INF-RF2: INF-RF2
- 83 Pond INF-RF3: INF-RF3
- 84 Pond INF-RF4: INF-RF4
- 85 Pond INF-RF5: INF-RF5
- 85 Pond INF-RF6: INF-RF6
- 86 Link DP-1: DESIGN POINT 1
- 86 Link DP-2: DESIGN POINT 2

85 West Street Northborough JN-1207.03 - Proposed

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 26

Rainfall Events Listing (selected events)

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-year	Type III 24-hr		Default	24.00	1	3.28	2
2	10-year	Type III 24-hr		Default	24.00	1	5.02	2
3	100-year	Type III 24-hr		Default	24.00	1	7.78	2

85 West Street Northborough JN-1207.03 - Proposed

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 27

Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
57,233	39	>75% Grass cover, Good, HSG A (PDA-100, PDA-101, PDA-102, PDA-200)
168,545	74	>75% Grass cover, Good, HSG C (PDA-100, PDA-101, PDA-102, PDA-103, PDA-104, PDA-105, PDA-106, PDA-200, PDA-201, PDA-202)
9,399	80	>75% Grass cover, Good, HSG D (PDA-102, PDA-200)
8,556	98	Paved parking, HSG A (PDA-100, PDA-101, PDA-102, PDA-200)
36,995	98	Paved parking, HSG C (PDA-101, PDA-102, PDA-103, PDA-104, PDA-105, PDA-106, PDA-200, PDA-201, PDA-202)
420	98	Paved parking, HSG D (PDA-100, PDA-102, PDA-200)
16,716	98	Roofs, HSG C (PDA-102, PDA-200, RF-02, RF-03, RF-04, RF-05, RF-06)
11,813	98	Water Surface, HSG C (PDA-200)
5,791	98	Water Surface, HSG D (PDA-200)
25,377	55	Woods, Good, HSG B (PDA-200)
387,792	70	Woods, Good, HSG C (PDA-100, PDA-102, PDA-104, PDA-105, PDA-106, PDA-200, PDA-201, PDA-202)
25,648	77	Woods, Good, HSG D (PDA-200)
754,285	71	TOTAL AREA

85 West Street Northborough JN-1207.03 - Proposed

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 28

Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
65,789	HSG A	PDA-100, PDA-101, PDA-102, PDA-200
25,377	HSG B	PDA-200
621,861	HSG C	PDA-100, PDA-101, PDA-102, PDA-103, PDA-104, PDA-105, PDA-106, PDA-200, PDA-201, PDA-202, RF-02, RF-03, RF-04, RF-05, RF-06
41,258	HSG D	PDA-100, PDA-102, PDA-200
0	Other	
754,285		TOTAL AREA

85 West Street Northborough JN-1207.03 - Proposed

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 29

Ground Covers (all nodes)

HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover
57,233	0	168,545	9,399	0	235,177	>75% Grass cover, Good
8,556	0	36,995	420	0	45,971	Paved parking
0	0	16,716	0	0	16,716	Roofs
0	0	11,813	5,791	0	17,604	Water Surface
0	25,377	387,792	25,648	0	438,817	Woods, Good
65,789	25,377	621,861	41,258	0	754,285	TOTAL AREA

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 30

Time span=0.00-96.00 hrs, dt=0.01 hrs, 9601 points x 2
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentPDA-100: PDA-100	Runoff Area=44,067 sf 6.77% Impervious Runoff Depth=0.05" Flow Length=263' Tc=15.1 min CN=45 Runoff=0.0 cfs 196 cf
SubcatchmentPDA-101: PDA-101	Runoff Area=8,231 sf 34.50% Impervious Runoff Depth=0.44" Flow Length=178' Tc=6.0 min CN=60 Runoff=0.1 cfs 302 cf
SubcatchmentPDA-102: PDA-102	Runoff Area=64,057 sf 11.83% Impervious Runoff Depth=0.88" Flow Length=618' Tc=15.8 min CN=70 Runoff=1.0 cfs 4,671 cf
SubcatchmentPDA-103: PDA-103	Runoff Area=7,866 sf 52.29% Impervious Runoff Depth=1.99" Flow Length=356' Tc=6.0 min CN=87 Runoff=0.4 cfs 1,302 cf
SubcatchmentPDA-104: PDA-104	Runoff Area=60,851 sf 4.61% Impervious Runoff Depth=1.03" Flow Length=889' Tc=17.5 min CN=73 Runoff=1.1 cfs 5,245 cf
SubcatchmentPDA-105: PDA-105	Runoff Area=92,080 sf 9.15% Impervious Runoff Depth=1.03" Flow Length=797' Tc=21.7 min CN=73 Runoff=1.6 cfs 7,937 cf
SubcatchmentPDA-106: PDA-106	Runoff Area=22,980 sf 57.09% Impervious Runoff Depth=2.07" Flow Length=489' Slope=0.0460 '/' Tc=7.2 min CN=88 Runoff=1.2 cfs 3,962 cf
SubcatchmentPDA-200: PDA-200	Runoff Area=313,513 sf 6.86% Impervious Runoff Depth=0.98" Flow Length=966' Tc=20.0 min CN=72 Runoff=5.1 cfs 25,595 cf
SubcatchmentPDA-201: Subcat PDA-201	Runoff Area=15,861 sf 17.40% Impervious Runoff Depth=1.33" Flow Length=228' Tc=6.0 min CN=78 Runoff=0.6 cfs 1,761 cf
SubcatchmentPDA-202: Subcat PDA-202	Runoff Area=110,630 sf 0.01% Impervious Runoff Depth=0.93" Flow Length=830' Tc=22.6 min CN=71 Runoff=1.6 cfs 8,542 cf
SubcatchmentRF-02: RF-02	Runoff Area=3,116 sf 100.00% Impervious Runoff Depth=3.05" Tc=0.0 min CN=98 Runoff=0.3 cfs 791 cf
SubcatchmentRF-03: RF-03	Runoff Area=1,685 sf 100.00% Impervious Runoff Depth=3.05" Tc=0.0 min CN=98 Runoff=0.2 cfs 428 cf
SubcatchmentRF-04: RF-04	Runoff Area=3,116 sf 100.00% Impervious Runoff Depth=3.05" Tc=0.0 min CN=98 Runoff=0.3 cfs 791 cf
SubcatchmentRF-05: RF-05	Runoff Area=3,116 sf 100.00% Impervious Runoff Depth=3.05" Tc=0.0 min CN=98 Runoff=0.3 cfs 791 cf
SubcatchmentRF-06: RF-06	Runoff Area=3,116 sf 100.00% Impervious Runoff Depth=3.05" Tc=0.0 min CN=98 Runoff=0.3 cfs 791 cf
Pond 1P: Pond	Peak Elev=320.14' Storage=44 cf Inflow=1.3 cfs 6,617 cf Outflow=1.3 cfs 6,617 cf

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 31

Pond INF-103: INF-103 Peak Elev=326.75' Storage=180 cf Inflow=0.4 cfs 1,503 cf
Discarded=0.0 cfs 292 cf Primary=0.4 cfs 1,211 cf Outflow=0.4 cfs 1,503 cf

Pond INF-104: INF-104 Peak Elev=326.79' Storage=2,155 cf Inflow=1.2 cfs 6,036 cf
Discarded=0.1 cfs 5,604 cf Primary=0.1 cfs 433 cf Outflow=0.2 cfs 6,036 cf

Pond INF-105: INF-105 Peak Elev=348.05' Storage=4,128 cf Inflow=1.6 cfs 7,937 cf
Discarded=0.1 cfs 7,937 cf Primary=0.0 cfs 0 cf Outflow=0.1 cfs 7,937 cf

Pond INF-106: INF-106 Peak Elev=355.62' Storage=2,343 cf Inflow=1.2 cfs 3,962 cf
Discarded=0.0 cfs 3,761 cf Primary=0.0 cfs 201 cf Outflow=0.1 cfs 3,962 cf

Pond INF-201: INF-201 Peak Elev=337.80' Storage=3,507 cf Inflow=1.9 cfs 10,664 cf
Discarded=0.2 cfs 8,070 cf Primary=0.6 cfs 2,593 cf Outflow=0.7 cfs 10,664 cf

Pond INF-RF2: INF-RF2 Peak Elev=335.70' Storage=116 cf Inflow=0.3 cfs 791 cf
Discarded=0.0 cfs 431 cf Primary=0.3 cfs 360 cf Outflow=0.3 cfs 791 cf

Pond INF-RF3: INF-RF3 Peak Elev=333.57' Storage=106 cf Inflow=0.2 cfs 428 cf
Discarded=0.0 cfs 333 cf Primary=0.1 cfs 95 cf Outflow=0.1 cfs 428 cf

Pond INF-RF4: INF-RF4 Peak Elev=0.00' Storage=0 cf
Discarded=0.0 cfs 0 cf Primary=0.0 cfs 0 cf

Pond INF-RF5: INF-RF5 Peak Elev=377.70' Storage=116 cf Inflow=0.3 cfs 791 cf
Discarded=0.0 cfs 431 cf Primary=0.3 cfs 360 cf Outflow=0.3 cfs 791 cf

Pond INF-RF6: INF-RF6 Peak Elev=377.70' Storage=116 cf Inflow=0.3 cfs 791 cf
Discarded=0.0 cfs 431 cf Primary=0.3 cfs 360 cf Outflow=0.3 cfs 791 cf

Link DP-1: DESIGN POINT 1 Inflow=1.3 cfs 6,813 cf
Primary=1.3 cfs 6,813 cf

Link DP-2: DESIGN POINT 2 Inflow=5.3 cfs 29,004 cf
Primary=5.3 cfs 29,004 cf

Total Runoff Area = 754,285 sf Runoff Volume = 63,105 cf Average Runoff Depth = 1.00"
89.36% Pervious = 673,994 sf 10.64% Impervious = 80,291 sf

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 32

Summary for Subcatchment PDA-100: PDA-100

Runoff = 0.0 cfs @ 15.32 hrs, Volume= 196 cf, Depth= 0.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
2,894	98	Paved parking, HSG A
37,895	39	>75% Grass cover, Good, HSG A
395	70	Woods, Good, HSG C
2,795	74	>75% Grass cover, Good, HSG C
88	98	Paved parking, HSG D
44,067	45	Weighted Average
41,085		93.23% Pervious Area
2,982		6.77% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.2	50	0.0256	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
3.9	213	0.0170	0.91		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
15.1	263	Total			

Summary for Subcatchment PDA-101: PDA-101

Runoff = 0.1 cfs @ 12.12 hrs, Volume= 302 cf, Depth= 0.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
5,156	39	>75% Grass cover, Good, HSG A
2,334	98	Paved parking, HSG A
235	74	>75% Grass cover, Good, HSG C
506	98	Paved parking, HSG C
8,231	60	Weighted Average
5,391		65.50% Pervious Area
2,840		34.50% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.4	50	0.0900	2.19		Sheet Flow, Smooth surfaces n= 0.011 P2= 3.20"
0.4	128	0.1076	5.28		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
0.8	178	Total, Increased to minimum Tc = 6.0 min			

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 33

Summary for Subcatchment PDA-102: PDA-102

Runoff = 1.0 cfs @ 12.24 hrs, Volume= 4,671 cf, Depth= 0.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
11,117	39	>75% Grass cover, Good, HSG A
3,051	98	Paved parking, HSG A
19,777	70	Woods, Good, HSG C
25,414	74	>75% Grass cover, Good, HSG C
2,188	98	Paved parking, HSG C
2,272	98	Roofs, HSG C
173	80	>75% Grass cover, Good, HSG D
66	98	Paved parking, HSG D
64,057	70	Weighted Average
56,481		88.17% Pervious Area
7,576		11.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.1	50	0.0908	0.27		Sheet Flow, Grass: Short n= 0.150 P2= 3.20"
8.5	360	0.0200	0.71		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
4.2	208	0.0240	0.82	4.10	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00' n= 0.200
15.8	618	Total			

Summary for Subcatchment PDA-103: PDA-103

Runoff = 0.4 cfs @ 12.09 hrs, Volume= 1,302 cf, Depth= 1.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
3,753	74	>75% Grass cover, Good, HSG C
4,113	98	Paved parking, HSG C
0	70	Woods, Good, HSG C
7,866	87	Weighted Average
3,753		47.71% Pervious Area
4,113		52.29% Impervious Area

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 34

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.3	36	0.0611	1.75		Sheet Flow, Smooth surfaces n= 0.011 P2= 3.20"
0.4	188	0.0189	8.17	14.44	Pipe Channel, 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013
0.2	132	0.0902	10.59	52.95	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00' n= 0.030 Short grass
0.9	356	Total, Increased to minimum Tc = 6.0 min			

Summary for Subcatchment PDA-104: PDA-104

Runoff = 1.1 cfs @ 12.27 hrs, Volume= 5,245 cf, Depth= 1.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
19,407	74	>75% Grass cover, Good, HSG C
38,640	70	Woods, Good, HSG C
2,804	98	Paved parking, HSG C
60,851	73	Weighted Average
58,047		95.39% Pervious Area
2,804		4.61% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.2	50	0.0324	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
4.3	335	0.0689	1.31		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.1	151	0.1167	2.39		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1.6	131	0.0730	1.35		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.3	222	0.1039	11.16	3.90	Pipe Channel, 8.0" Round Area= 0.3 sf Perim= 2.1' r= 0.17' n= 0.013
17.5	889	Total			

Summary for Subcatchment PDA-105: PDA-105

Runoff = 1.6 cfs @ 12.32 hrs, Volume= 7,937 cf, Depth= 1.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 35

Area (sf)	CN	Description
65,985	70	Woods, Good, HSG C
8,422	98	Paved parking, HSG C
17,673	74	>75% Grass cover, Good, HSG C
92,080	73	Weighted Average
83,658		90.85% Pervious Area
8,422		9.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.0	50	0.0176	0.06		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
5.0	347	0.0528	1.15		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.1	30	0.3300	4.02		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.8	208	0.0684	4.21		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
2.8	162	0.0333	0.97	4.83	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 ' Top.W=8.00' n= 0.200
21.7	797	Total			

Summary for Subcatchment PDA-106: PDA-106

Runoff = 1.2 cfs @ 12.10 hrs, Volume= 3,962 cf, Depth= 2.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
9,097	74	>75% Grass cover, Good, HSG C
13,120	98	Paved parking, HSG C
763	70	Woods, Good, HSG C
22,980	88	Weighted Average
9,860		42.91% Pervious Area
13,120		57.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.2	489	0.0460	1.13	5.67	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 ' Top.W=8.00' n= 0.200

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 36

Summary for Subcatchment PDA-200: PDA-200

Runoff = 5.1 cfs @ 12.30 hrs, Volume= 25,595 cf, Depth= 0.98"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
25,377	55	Woods, Good, HSG B
36,942	74	>75% Grass cover, Good, HSG C
191,740	70	Woods, Good, HSG C
3,065	39	>75% Grass cover, Good, HSG A
295	98	Roofs, HSG C
3,073	98	Paved parking, HSG C
277	98	Paved parking, HSG A
266	98	Paved parking, HSG D
11,813	98	Water Surface, HSG C
5,791	98	Water Surface, HSG D
25,648	77	Woods, Good, HSG D
9,226	80	>75% Grass cover, Good, HSG D
313,513	72	Weighted Average
291,998		93.14% Pervious Area
21,515		6.86% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	50	0.0296	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
3.4	247	0.0587	1.21		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
2.6	248	0.0524	1.60		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1.8	207	0.1498	1.94		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.4	87	0.0274	3.36		Shallow Concentrated Flow, Paved Kv= 20.3 fps
0.1	16	0.3333	4.04		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1.2	111	0.0901	1.50		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
20.0	966	Total			

Summary for Subcatchment PDA-201: Subcat PDA-201

Runoff = 0.6 cfs @ 12.09 hrs, Volume= 1,761 cf, Depth= 1.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 37

Area (sf)	CN	Description
12,163	74	>75% Grass cover, Good, HSG C
938	70	Woods, Good, HSG C
2,760	98	Paved parking, HSG C
15,861	78	Weighted Average
13,101		82.60% Pervious Area
2,760		17.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	31	0.0200	1.09		Sheet Flow, Smooth surfaces n= 0.011 P2= 3.20"
0.3	197	0.0609	13.05	65.26	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00' n= 0.020
0.8	228	Total, Increased to minimum Tc = 6.0 min			

Summary for Subcatchment PDA-202: Subcat PDA-202

Runoff = 1.6 cfs @ 12.35 hrs, Volume= 8,542 cf, Depth= 0.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
41,066	74	>75% Grass cover, Good, HSG C
10	98	Paved parking, HSG C
69,554	70	Woods, Good, HSG C
110,630	71	Weighted Average
110,620		99.99% Pervious Area
10		0.01% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.3	50	0.0250	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
9.2	308	0.0500	0.56		Shallow Concentrated Flow, Forest w/Heavy Litter Kv= 2.5 fps
1.7	169	0.0592	1.70		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.4	303	0.0460	11.34	56.72	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00' n= 0.020
22.6	830	Total			

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 38

Summary for Subcatchment RF-02: RF-02

Runoff = 0.3 cfs @ 12.00 hrs, Volume= 791 cf, Depth= 3.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
3,116	98	Roofs, HSG C
3,116		100.00% Impervious Area

Summary for Subcatchment RF-03: RF-03

Runoff = 0.2 cfs @ 12.00 hrs, Volume= 428 cf, Depth= 3.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
1,685	98	Roofs, HSG C
1,685		100.00% Impervious Area

Summary for Subcatchment RF-04: RF-04

Runoff = 0.3 cfs @ 12.00 hrs, Volume= 791 cf, Depth= 3.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
3,116	98	Roofs, HSG C
3,116		100.00% Impervious Area

Summary for Subcatchment RF-05: RF-05

Runoff = 0.3 cfs @ 12.00 hrs, Volume= 791 cf, Depth= 3.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
3,116	98	Roofs, HSG C
3,116		100.00% Impervious Area

Summary for Subcatchment RF-06: RF-06

Runoff = 0.3 cfs @ 12.00 hrs, Volume= 791 cf, Depth= 3.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Type III 24-hr 2-year Rainfall=3.28"

Area (sf)	CN	Description
3,116	98	Roofs, HSG C
3,116		100.00% Impervious Area

Summary for Pond 1P: Pond

Inflow Area = 259,181 sf, 16.20% Impervious, Inflow Depth = 0.31" for 2-year event
 Inflow = 1.3 cfs @ 12.20 hrs, Volume= 6,617 cf
 Outflow = 1.3 cfs @ 12.21 hrs, Volume= 6,617 cf, Atten= 0%, Lag= 0.6 min
 Primary = 1.3 cfs @ 12.21 hrs, Volume= 6,617 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 320.14' @ 12.21 hrs Surf.Area= 350 sf Storage= 44 cf

Plug-Flow detention time= 1.2 min calculated for 6,617 cf (100% of inflow)
 Center-of-Mass det. time= 0.9 min (872.0 - 871.1)

Volume	Invert	Avail.Storage	Storage Description
#1	320.00'	1,892 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
320.00	269	0	0
321.00	841	555	555
322.00	1,833	1,337	1,892

Device	Routing	Invert	Outlet Devices
#1	Primary	320.20'	8.0" Round Culvert L= 15.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 320.20' / 320.00' S= 0.0133 '/ Cc= 0.900 n= 0.013, Flow Area= 0.35 sf
#2	Primary	320.00'	10.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

Primary OutFlow Max=1.3 cfs @ 12.21 hrs HW=320.14' TW=0.00' (Dynamic Tailwater)

- ↑ 1=Culvert (Controls 0.0 cfs)
- ↓ 2=Broad-Crested Rectangular Weir(Weir Controls 1.3 cfs @ 0.91 fps)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 40

Summary for Pond INF-103: INF-103

Inflow Area = 30,846 sf, 55.87% Impervious, Inflow Depth = 0.58" for 2-year event
 Inflow = 0.4 cfs @ 12.09 hrs, Volume= 1,503 cf
 Outflow = 0.4 cfs @ 12.11 hrs, Volume= 1,503 cf, Atten= 4%, Lag= 1.4 min
 Discarded = 0.0 cfs @ 9.24 hrs, Volume= 292 cf
 Primary = 0.4 cfs @ 12.11 hrs, Volume= 1,211 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 326.75' @ 12.11 hrs Surf.Area= 473 sf Storage= 180 cf

Plug-Flow detention time= 93.1 min calculated for 1,503 cf (100% of inflow)
 Center-of-Mass det. time= 93.2 min (924.2 - 831.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	326.00'	310 cf	12.42'W x 38.07'L x 2.00'H Field A 945 cf Overall - 171 cf Embedded = 774 cf x 40.0% Voids
#2A	326.50'	171 cf	ADS_StormTech SC-160LP +Cap x 25 Inside #1 Effective Size= 18.0"W x 12.0"H => 0.96 sf x 7.12'L = 6.8 cf Overall Size= 25.0"W x 12.0"H x 7.56'L with 0.44' Overlap 25 Chambers in 5 Rows
		481 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	326.56'	6.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads
#2	Discarded	326.00'	0.276 in/hr Exfiltration - Loamy Sand over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.0 cfs @ 9.24 hrs HW=326.02' (Free Discharge)

↑**2=Exfiltration - Loamy Sand** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=0.4 cfs @ 12.11 hrs HW=326.75' TW=320.12' (Dynamic Tailwater)

↑**1=Orifice/Grate** (Orifice Controls 0.4 cfs @ 1.48 fps)

Summary for Pond INF-104: INF-104

Inflow Area = 156,047 sf, 9.19% Impervious, Inflow Depth = 0.46" for 2-year event
 Inflow = 1.2 cfs @ 12.26 hrs, Volume= 6,036 cf
 Outflow = 0.2 cfs @ 13.09 hrs, Volume= 6,036 cf, Atten= 80%, Lag= 50.2 min
 Discarded = 0.1 cfs @ 11.84 hrs, Volume= 5,604 cf
 Primary = 0.1 cfs @ 13.09 hrs, Volume= 433 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 326.79' @ 13.09 hrs Surf.Area= 1,889 sf Storage= 2,155 cf

Plug-Flow detention time= 138.3 min calculated for 6,036 cf (100% of inflow)
 Center-of-Mass det. time= 138.3 min (996.4 - 858.2)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 41

Volume	Invert	Avail.Storage	Storage Description
#1A	325.10'	1,726 cf	25.25'W x 74.82'L x 3.50'H Field A 6,612 cf Overall - 2,297 cf Embedded = 4,315 cf x 40.0% Voids
#2A	325.60'	2,297 cf	ADS_StormTech SC-740 +Cap x 50 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 50 Chambers in 5 Rows
		4,023 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	327.14'	6.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads
#2	Primary	326.81'	10.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	326.64'	12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	325.10'	3.072 in/hr Exfiltration - Loamy Sand over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.1 cfs @ 11.84 hrs HW=325.14' (Free Discharge)

↳ **4=Exfiltration - Loamy Sand** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=0.1 cfs @ 13.09 hrs HW=326.79' TW=320.06' (Dynamic Tailwater)

↳ **1=Orifice/Grate** (Controls 0.0 cfs)

↳ **2=Orifice/Grate** (Controls 0.0 cfs)

↳ **3=Orifice/Grate** (Orifice Controls 0.1 cfs @ 1.34 fps)

Summary for Pond INF-105: INF-105

Inflow Area = 92,080 sf, 9.15% Impervious, Inflow Depth = 1.03" for 2-year event
 Inflow = 1.6 cfs @ 12.32 hrs, Volume= 7,937 cf
 Outflow = 0.1 cfs @ 11.95 hrs, Volume= 7,937 cf, Atten= 93%, Lag= 0.0 min
 Discarded = 0.1 cfs @ 11.95 hrs, Volume= 7,937 cf
 Primary = 0.0 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 348.05' @ 16.17 hrs Surf.Area= 3,941 sf Storage= 4,128 cf

Plug-Flow detention time= 380.0 min calculated for 7,937 cf (100% of inflow)

Center-of-Mass det. time= 380.0 min (1,258.3 - 878.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	346.50'	3,532 cf	44.25'W x 89.06'L x 3.50'H Field A 13,793 cf Overall - 4,962 cf Embedded = 8,831 cf x 40.0% Voids
#2A	347.00'	4,962 cf	ADS_StormTech SC-740 +Cap x 108 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 108 Chambers in 9 Rows
		8,494 cf	Total Available Storage

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 42

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	348.54'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	348.38'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	348.21'	10.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	346.50'	1.284 in/hr Exfiltration - Sandy Loam over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.1 cfs @ 11.95 hrs HW=346.54' (Free Discharge)↑**4=Exfiltration - Sandy Loam** (Exfiltration Controls 0.1 cfs)**Primary OutFlow** Max=0.0 cfs @ 0.00 hrs HW=346.50' TW=325.10' (Dynamic Tailwater)↑**1=Orifice/Grate** (Controls 0.0 cfs)|**2=Orifice/Grate** (Controls 0.0 cfs)|**3=Orifice/Grate** (Controls 0.0 cfs)**Summary for Pond INF-106: INF-106**

Inflow Area =	22,980 sf, 57.09% Impervious, Inflow Depth = 2.07" for 2-year event
Inflow =	1.2 cfs @ 12.10 hrs, Volume= 3,962 cf
Outflow =	0.1 cfs @ 14.63 hrs, Volume= 3,962 cf, Atten= 95%, Lag= 151.4 min
Discarded =	0.0 cfs @ 10.74 hrs, Volume= 3,761 cf
Primary =	0.0 cfs @ 14.63 hrs, Volume= 201 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 355.62' @ 14.63 hrs Surf.Area= 786 sf Storage= 2,343 cf

Plug-Flow detention time= 671.0 min calculated for 3,962 cf (100% of inflow)

Center-of-Mass det. time= 671.0 min (1,486.2 - 815.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	351.25'	1,389 cf	28.50'W x 27.59'L x 6.75'H Field A 5,308 cf Overall - 1,834 cf Embedded = 3,474 cf x 40.0% Voids
#2A	352.00'	1,834 cf	ADS_StormTech MC-4500 b +Cap x 15 Inside #1 Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 15 Chambers in 3 Rows Cap Storage= +39.5 cf x 2 x 3 rows = 237.0 cf
		3,224 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	355.55'	6.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads
#2	Discarded	351.25'	1.758 in/hr Exfiltration - Sandy Loam over Surface area Phase-In= 0.01'

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 43

Discarded OutFlow Max=0.0 cfs @ 10.74 hrs HW=351.32' (Free Discharge)↑**2=Exfiltration - Sandy Loam** (Exfiltration Controls 0.0 cfs)**Primary OutFlow** Max=0.0 cfs @ 14.63 hrs HW=355.62' TW=326.63' (Dynamic Tailwater)↑**1=Orifice/Grate** (Orifice Controls 0.0 cfs @ 0.93 fps)**Summary for Pond INF-201: INF-201**

Inflow Area = 129,607 sf, 4.54% Impervious, Inflow Depth = 0.99" for 2-year event
 Inflow = 1.9 cfs @ 12.33 hrs, Volume= 10,664 cf
 Outflow = 0.7 cfs @ 12.83 hrs, Volume= 10,664 cf, Atten= 61%, Lag= 30.2 min
 Discarded = 0.2 cfs @ 11.84 hrs, Volume= 8,070 cf
 Primary = 0.6 cfs @ 12.83 hrs, Volume= 2,593 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 337.80' @ 12.83 hrs Surf.Area= 3,236 sf Storage= 3,507 cf

Plug-Flow detention time= 147.5 min calculated for 10,663 cf (100% of inflow)

Center-of-Mass det. time= 147.5 min (1,022.2 - 874.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	336.20'	2,914 cf	39.50'W x 81.94'L x 3.50'H Field A 11,328 cf Overall - 4,043 cf Embedded = 7,285 cf x 40.0% Voids
#2A	336.70'	4,043 cf	ADS_StormTech SC-740 +Cap x 88 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 88 Chambers in 8 Rows
		6,957 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	337.45'	15.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	338.24'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Discarded	336.20'	2.262 in/hr Exfiltration - Sandy Loam over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.2 cfs @ 11.84 hrs HW=336.24' (Free Discharge)↑**3=Exfiltration - Sandy Loam** (Exfiltration Controls 0.2 cfs)**Primary OutFlow** Max=0.6 cfs @ 12.83 hrs HW=337.80' TW=0.00' (Dynamic Tailwater)↑**1=Orifice/Grate** (Orifice Controls 0.6 cfs @ 2.02 fps)↑**2=Orifice/Grate** (Controls 0.0 cfs)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 44

Summary for Pond INF-RF2: INF-RF2

Inflow Area = 3,116 sf, 100.00% Impervious, Inflow Depth = 3.05" for 2-year event
 Inflow = 0.3 cfs @ 12.00 hrs, Volume= 791 cf
 Outflow = 0.3 cfs @ 12.01 hrs, Volume= 791 cf, Atten= 5%, Lag= 0.7 min
 Discarded = 0.0 cfs @ 8.99 hrs, Volume= 431 cf
 Primary = 0.3 cfs @ 12.01 hrs, Volume= 360 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 335.70' @ 12.01 hrs Surf.Area= 118 sf Storage= 116 cf

Plug-Flow detention time= 69.9 min calculated for 791 cf (100% of inflow)
 Center-of-Mass det. time= 69.9 min (820.2 - 750.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	334.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	334.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	334.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	335.38'	8.0" Round Culvert L= 52.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.38' / 333.00' S= 0.0458 '/' Cc= 0.900 n= 0.013, Flow Area= 0.35 sf

Discarded OutFlow Max=0.0 cfs @ 8.99 hrs HW=334.04' (Free Discharge)
 ↑1=Exfiltration (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=0.3 cfs @ 12.01 hrs HW=335.70' TW=0.00' (Dynamic Tailwater)
 ↑2=Culvert (Inlet Controls 0.3 cfs @ 1.53 fps)

Summary for Pond INF-RF3: INF-RF3

Inflow Area = 1,685 sf, 100.00% Impervious, Inflow Depth = 3.05" for 2-year event
 Inflow = 0.2 cfs @ 12.00 hrs, Volume= 428 cf
 Outflow = 0.1 cfs @ 12.06 hrs, Volume= 428 cf, Atten= 34%, Lag= 3.5 min
 Discarded = 0.0 cfs @ 10.71 hrs, Volume= 333 cf
 Primary = 0.1 cfs @ 12.06 hrs, Volume= 95 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 333.57' @ 12.06 hrs Surf.Area= 118 sf Storage= 106 cf

Plug-Flow detention time= 84.3 min calculated for 428 cf (100% of inflow)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 45

Center-of-Mass det. time= 84.3 min (834.7 - 750.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	332.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	332.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	332.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	333.38'	8.0" Round Culvert L= 57.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 333.38' / 331.00' S= 0.0418 '/ Cc= 0.900 n= 0.013, Flow Area= 0.35 sf

Discarded OutFlow Max=0.0 cfs @ 10.71 hrs HW=332.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.0 cfs)**Primary OutFlow** Max=0.1 cfs @ 12.06 hrs HW=333.57' TW=0.00' (Dynamic Tailwater)↑**2=Culvert** (Inlet Controls 0.1 cfs @ 1.16 fps)**Summary for Pond INF-RF4: INF-RF4**

Volume	Invert	Avail.Storage	Storage Description
#1A	366.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	366.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	366.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	367.38'	8.0" Round Culvert L= 37.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 367.38' / 366.00' S= 0.0373 '/ Cc= 0.900 n= 0.013, Flow Area= 0.35 sf

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 46

Discarded OutFlow Max=0.0 cfs @ 0.00 hrs HW=0.00' (Free Discharge)

↑**1=Exfiltration** (Controls 0.0 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=0.00' (Free Discharge)

↑**2=Culvert** (Controls 0.0 cfs)

Summary for Pond INF-RF5: INF-RF5

Inflow Area = 3,116 sf, 100.00% Impervious, Inflow Depth = 3.05" for 2-year event
 Inflow = 0.3 cfs @ 12.00 hrs, Volume= 791 cf
 Outflow = 0.3 cfs @ 12.01 hrs, Volume= 791 cf, Atten= 5%, Lag= 0.7 min
 Discarded = 0.0 cfs @ 8.99 hrs, Volume= 431 cf
 Primary = 0.3 cfs @ 12.01 hrs, Volume= 360 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 377.70' @ 12.01 hrs Surf.Area= 118 sf Storage= 116 cf

Plug-Flow detention time= 69.9 min calculated for 791 cf (100% of inflow)

Center-of-Mass det. time= 69.9 min (820.2 - 750.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	376.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	376.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	377.38'	8.0" Round Culvert L= 16.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 377.38' / 376.00' S= 0.0862 '/' Cc= 0.900 n= 0.013, Flow Area= 0.35 sf
#2	Discarded	376.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.0 cfs @ 8.99 hrs HW=376.04' (Free Discharge)

↑**2=Exfiltration** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=0.3 cfs @ 12.01 hrs HW=377.70' TW=336.43' (Dynamic Tailwater)

↑**1=Culvert** (Inlet Controls 0.3 cfs @ 1.53 fps)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 2-year Rainfall=3.28"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 47

Summary for Pond INF-RF6: INF-RF6

Inflow Area = 3,116 sf, 100.00% Impervious, Inflow Depth = 3.05" for 2-year event
 Inflow = 0.3 cfs @ 12.00 hrs, Volume= 791 cf
 Outflow = 0.3 cfs @ 12.01 hrs, Volume= 791 cf, Atten= 5%, Lag= 0.7 min
 Discarded = 0.0 cfs @ 8.99 hrs, Volume= 431 cf
 Primary = 0.3 cfs @ 12.01 hrs, Volume= 360 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 377.70' @ 12.01 hrs Surf.Area= 118 sf Storage= 116 cf

Plug-Flow detention time= 69.9 min calculated for 791 cf (100% of inflow)
 Center-of-Mass det. time= 69.9 min (820.2 - 750.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	376.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	376.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	377.38'	8.0" Round Culvert L= 58.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 377.38' / 376.00' S= 0.0238 '/' Cc= 0.900 n= 0.013, Flow Area= 0.35 sf
#2	Discarded	376.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.0 cfs @ 8.99 hrs HW=376.04' (Free Discharge)
 ↑**2=Exfiltration** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=0.3 cfs @ 12.01 hrs HW=377.70' TW=0.00' (Dynamic Tailwater)
 ↑**1=Culvert** (Inlet Controls 0.3 cfs @ 1.53 fps)

Summary for Link DP-1: DESIGN POINT 1

Inflow Area = 303,248 sf, 14.83% Impervious, Inflow Depth = 0.27" for 2-year event
 Inflow = 1.3 cfs @ 12.21 hrs, Volume= 6,813 cf
 Primary = 1.3 cfs @ 12.21 hrs, Volume= 6,813 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Summary for Link DP-2: DESIGN POINT 2

Inflow Area = 451,037 sf, 7.83% Impervious, Inflow Depth = 0.77" for 2-year event
Inflow = 5.3 cfs @ 12.29 hrs, Volume= 29,004 cf
Primary = 5.3 cfs @ 12.29 hrs, Volume= 29,004 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 49

Time span=0.00-96.00 hrs, dt=0.01 hrs, 9601 points x 2
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentPDA-100: PDA-100	Runoff Area=44,067 sf 6.77% Impervious Runoff Depth=0.45" Flow Length=263' Tc=15.1 min CN=45 Runoff=0.2 cfs 1,646 cf
SubcatchmentPDA-101: PDA-101	Runoff Area=8,231 sf 34.50% Impervious Runoff Depth=1.31" Flow Length=178' Tc=6.0 min CN=60 Runoff=0.3 cfs 900 cf
SubcatchmentPDA-102: PDA-102	Runoff Area=64,057 sf 11.83% Impervious Runoff Depth=2.05" Flow Length=618' Tc=15.8 min CN=70 Runoff=2.6 cfs 10,949 cf
SubcatchmentPDA-103: PDA-103	Runoff Area=7,866 sf 52.29% Impervious Runoff Depth=3.59" Flow Length=356' Tc=6.0 min CN=87 Runoff=0.7 cfs 2,351 cf
SubcatchmentPDA-104: PDA-104	Runoff Area=60,851 sf 4.61% Impervious Runoff Depth=2.30" Flow Length=889' Tc=17.5 min CN=73 Runoff=2.6 cfs 11,644 cf
SubcatchmentPDA-105: PDA-105	Runoff Area=92,080 sf 9.15% Impervious Runoff Depth=2.30" Flow Length=797' Tc=21.7 min CN=73 Runoff=3.7 cfs 17,619 cf
SubcatchmentPDA-106: PDA-106	Runoff Area=22,980 sf 57.09% Impervious Runoff Depth=3.69" Flow Length=489' Slope=0.0460 '/' Tc=7.2 min CN=88 Runoff=2.1 cfs 7,062 cf
SubcatchmentPDA-200: PDA-200	Runoff Area=313,513 sf 6.86% Impervious Runoff Depth=2.21" Flow Length=966' Tc=20.0 min CN=72 Runoff=12.4 cfs 57,824 cf
SubcatchmentPDA-201: Subcat PDA-201	Runoff Area=15,861 sf 17.40% Impervious Runoff Depth=2.73" Flow Length=228' Tc=6.0 min CN=78 Runoff=1.2 cfs 3,607 cf
SubcatchmentPDA-202: Subcat PDA-202	Runoff Area=110,630 sf 0.01% Impervious Runoff Depth=2.13" Flow Length=830' Tc=22.6 min CN=71 Runoff=4.0 cfs 19,652 cf
SubcatchmentRF-02: RF-02	Runoff Area=3,116 sf 100.00% Impervious Runoff Depth=4.78" Tc=0.0 min CN=98 Runoff=0.4 cfs 1,242 cf
SubcatchmentRF-03: RF-03	Runoff Area=1,685 sf 100.00% Impervious Runoff Depth=4.78" Tc=0.0 min CN=98 Runoff=0.2 cfs 672 cf
SubcatchmentRF-04: RF-04	Runoff Area=3,116 sf 100.00% Impervious Runoff Depth=4.78" Tc=0.0 min CN=98 Runoff=0.4 cfs 1,242 cf
SubcatchmentRF-05: RF-05	Runoff Area=3,116 sf 100.00% Impervious Runoff Depth=4.78" Tc=0.0 min CN=98 Runoff=0.4 cfs 1,242 cf
SubcatchmentRF-06: RF-06	Runoff Area=3,116 sf 100.00% Impervious Runoff Depth=4.78" Tc=0.0 min CN=98 Runoff=0.4 cfs 1,242 cf
Pond 1P: Pond	Peak Elev=320.37' Storage=139 cf Inflow=5.8 cfs 29,307 cf Outflow=5.8 cfs 29,307 cf

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 50

Pond INF-103: INF-103 Peak Elev=326.96' Storage=249 cf Inflow=1.5 cfs 5,236 cf
Discarded=0.0 cfs 313 cf Primary=1.5 cfs 4,923 cf Outflow=1.5 cfs 5,236 cf

Pond INF-104: INF-104 Peak Elev=327.32' Storage=2,849 cf Inflow=2.8 cfs 20,262 cf
Discarded=0.1 cfs 7,727 cf Primary=2.6 cfs 12,534 cf Outflow=2.8 cfs 20,262 cf

Pond INF-105: INF-105 Peak Elev=348.79' Storage=6,206 cf Inflow=3.7 cfs 17,619 cf
Discarded=0.1 cfs 10,243 cf Primary=1.7 cfs 7,376 cf Outflow=1.8 cfs 17,619 cf

Pond INF-106: INF-106 Peak Elev=356.14' Storage=2,590 cf Inflow=2.1 cfs 7,062 cf
Discarded=0.0 cfs 4,178 cf Primary=1.1 cfs 2,885 cf Outflow=1.1 cfs 7,062 cf

Pond INF-201: INF-201 Peak Elev=338.46' Storage=5,013 cf Inflow=4.6 cfs 23,983 cf
Discarded=0.2 cfs 10,466 cf Primary=3.7 cfs 13,517 cf Outflow=3.9 cfs 23,983 cf

Pond INF-RF2: INF-RF2 Peak Elev=335.80' Storage=123 cf Inflow=0.4 cfs 1,242 cf
Discarded=0.0 cfs 518 cf Primary=0.4 cfs 724 cf Outflow=0.4 cfs 1,242 cf

Pond INF-RF3: INF-RF3 Peak Elev=333.67' Storage=113 cf Inflow=0.2 cfs 672 cf
Discarded=0.0 cfs 411 cf Primary=0.2 cfs 260 cf Outflow=0.2 cfs 672 cf

Pond INF-RF4: INF-RF4 Peak Elev=0.00' Storage=0 cf
Discarded=0.0 cfs 0 cf Primary=0.0 cfs 0 cf

Pond INF-RF5: INF-RF5 Peak Elev=377.80' Storage=123 cf Inflow=0.4 cfs 1,242 cf
Discarded=0.0 cfs 518 cf Primary=0.4 cfs 724 cf Outflow=0.4 cfs 1,242 cf

Pond INF-RF6: INF-RF6 Peak Elev=377.80' Storage=123 cf Inflow=0.4 cfs 1,242 cf
Discarded=0.0 cfs 518 cf Primary=0.4 cfs 724 cf Outflow=0.4 cfs 1,242 cf

Link DP-1: DESIGN POINT 1 Inflow=5.9 cfs 30,953 cf
Primary=5.9 cfs 30,953 cf

Link DP-2: DESIGN POINT 2 Inflow=15.3 cfs 73,050 cf
Primary=15.3 cfs 73,050 cf

Total Runoff Area = 754,285 sf Runoff Volume = 138,894 cf Average Runoff Depth = 2.21"
89.36% Pervious = 673,994 sf 10.64% Impervious = 80,291 sf

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 51

Summary for Subcatchment PDA-100: PDA-100

Runoff = 0.2 cfs @ 12.44 hrs, Volume= 1,646 cf, Depth= 0.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
2,894	98	Paved parking, HSG A
37,895	39	>75% Grass cover, Good, HSG A
395	70	Woods, Good, HSG C
2,795	74	>75% Grass cover, Good, HSG C
88	98	Paved parking, HSG D
44,067	45	Weighted Average
41,085		93.23% Pervious Area
2,982		6.77% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.2	50	0.0256	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
3.9	213	0.0170	0.91		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
15.1	263	Total			

Summary for Subcatchment PDA-101: PDA-101

Runoff = 0.3 cfs @ 12.10 hrs, Volume= 900 cf, Depth= 1.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
5,156	39	>75% Grass cover, Good, HSG A
2,334	98	Paved parking, HSG A
235	74	>75% Grass cover, Good, HSG C
506	98	Paved parking, HSG C
8,231	60	Weighted Average
5,391		65.50% Pervious Area
2,840		34.50% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.4	50	0.0900	2.19		Sheet Flow, Smooth surfaces n= 0.011 P2= 3.20"
0.4	128	0.1076	5.28		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
0.8	178	Total, Increased to minimum Tc = 6.0 min			

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 52

Summary for Subcatchment PDA-102: PDA-102

Runoff = 2.6 cfs @ 12.23 hrs, Volume= 10,949 cf, Depth= 2.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
11,117	39	>75% Grass cover, Good, HSG A
3,051	98	Paved parking, HSG A
19,777	70	Woods, Good, HSG C
25,414	74	>75% Grass cover, Good, HSG C
2,188	98	Paved parking, HSG C
2,272	98	Roofs, HSG C
173	80	>75% Grass cover, Good, HSG D
66	98	Paved parking, HSG D
64,057	70	Weighted Average
56,481		88.17% Pervious Area
7,576		11.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.1	50	0.0908	0.27		Sheet Flow, Grass: Short n= 0.150 P2= 3.20"
8.5	360	0.0200	0.71		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
4.2	208	0.0240	0.82	4.10	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00' n= 0.200
15.8	618	Total			

Summary for Subcatchment PDA-103: PDA-103

Runoff = 0.7 cfs @ 12.09 hrs, Volume= 2,351 cf, Depth= 3.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
3,753	74	>75% Grass cover, Good, HSG C
4,113	98	Paved parking, HSG C
0	70	Woods, Good, HSG C
7,866	87	Weighted Average
3,753		47.71% Pervious Area
4,113		52.29% Impervious Area

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 53

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.3	36	0.0611	1.75		Sheet Flow, Smooth surfaces n= 0.011 P2= 3.20"
0.4	188	0.0189	8.17	14.44	Pipe Channel, 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013
0.2	132	0.0902	10.59	52.95	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00' n= 0.030 Short grass
0.9	356	Total, Increased to minimum Tc = 6.0 min			

Summary for Subcatchment PDA-104: PDA-104

Runoff = 2.6 cfs @ 12.25 hrs, Volume= 11,644 cf, Depth= 2.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
19,407	74	>75% Grass cover, Good, HSG C
38,640	70	Woods, Good, HSG C
2,804	98	Paved parking, HSG C
60,851	73	Weighted Average
58,047		95.39% Pervious Area
2,804		4.61% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.2	50	0.0324	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
4.3	335	0.0689	1.31		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.1	151	0.1167	2.39		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1.6	131	0.0730	1.35		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.3	222	0.1039	11.16	3.90	Pipe Channel, 8.0" Round Area= 0.3 sf Perim= 2.1' r= 0.17' n= 0.013
17.5	889	Total			

Summary for Subcatchment PDA-105: PDA-105

Runoff = 3.7 cfs @ 12.32 hrs, Volume= 17,619 cf, Depth= 2.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 54

Area (sf)	CN	Description
65,985	70	Woods, Good, HSG C
8,422	98	Paved parking, HSG C
17,673	74	>75% Grass cover, Good, HSG C
92,080	73	Weighted Average
83,658		90.85% Pervious Area
8,422		9.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.0	50	0.0176	0.06		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
5.0	347	0.0528	1.15		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.1	30	0.3300	4.02		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.8	208	0.0684	4.21		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
2.8	162	0.0333	0.97	4.83	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 ' /' Top.W=8.00' n= 0.200
21.7	797	Total			

Summary for Subcatchment PDA-106: PDA-106

Runoff = 2.1 cfs @ 12.10 hrs, Volume= 7,062 cf, Depth= 3.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
9,097	74	>75% Grass cover, Good, HSG C
13,120	98	Paved parking, HSG C
763	70	Woods, Good, HSG C
22,980	88	Weighted Average
9,860		42.91% Pervious Area
13,120		57.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.2	489	0.0460	1.13	5.67	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 ' /' Top.W=8.00' n= 0.200

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 55

Summary for Subcatchment PDA-200: PDA-200

Runoff = 12.4 cfs @ 12.29 hrs, Volume= 57,824 cf, Depth= 2.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
25,377	55	Woods, Good, HSG B
36,942	74	>75% Grass cover, Good, HSG C
191,740	70	Woods, Good, HSG C
3,065	39	>75% Grass cover, Good, HSG A
295	98	Roofs, HSG C
3,073	98	Paved parking, HSG C
277	98	Paved parking, HSG A
266	98	Paved parking, HSG D
11,813	98	Water Surface, HSG C
5,791	98	Water Surface, HSG D
25,648	77	Woods, Good, HSG D
9,226	80	>75% Grass cover, Good, HSG D
313,513	72	Weighted Average
291,998		93.14% Pervious Area
21,515		6.86% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	50	0.0296	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
3.4	247	0.0587	1.21		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
2.6	248	0.0524	1.60		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1.8	207	0.1498	1.94		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.4	87	0.0274	3.36		Shallow Concentrated Flow, Paved Kv= 20.3 fps
0.1	16	0.3333	4.04		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1.2	111	0.0901	1.50		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
20.0	966	Total			

Summary for Subcatchment PDA-201: Subcat PDA-201

Runoff = 1.2 cfs @ 12.09 hrs, Volume= 3,607 cf, Depth= 2.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 56

Area (sf)	CN	Description
12,163	74	>75% Grass cover, Good, HSG C
938	70	Woods, Good, HSG C
2,760	98	Paved parking, HSG C
15,861	78	Weighted Average
13,101		82.60% Pervious Area
2,760		17.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	31	0.0200	1.09		Sheet Flow, Smooth surfaces n= 0.011 P2= 3.20"
0.3	197	0.0609	13.05	65.26	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 ' /' Top.W=8.00' n= 0.020
0.8	228	Total, Increased to minimum Tc = 6.0 min			

Summary for Subcatchment PDA-202: Subcat PDA-202

Runoff = 4.0 cfs @ 12.33 hrs, Volume= 19,652 cf, Depth= 2.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
41,066	74	>75% Grass cover, Good, HSG C
10	98	Paved parking, HSG C
69,554	70	Woods, Good, HSG C
110,630	71	Weighted Average
110,620		99.99% Pervious Area
10		0.01% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.3	50	0.0250	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
9.2	308	0.0500	0.56		Shallow Concentrated Flow, Forest w/Heavy Litter Kv= 2.5 fps
1.7	169	0.0592	1.70		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.4	303	0.0460	11.34	56.72	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 ' /' Top.W=8.00' n= 0.020
22.6	830	Total			

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 57

Summary for Subcatchment RF-02: RF-02

Runoff = 0.4 cfs @ 12.00 hrs, Volume= 1,242 cf, Depth= 4.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
3,116	98	Roofs, HSG C
3,116		100.00% Impervious Area

Summary for Subcatchment RF-03: RF-03

Runoff = 0.2 cfs @ 12.00 hrs, Volume= 672 cf, Depth= 4.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
1,685	98	Roofs, HSG C
1,685		100.00% Impervious Area

Summary for Subcatchment RF-04: RF-04

Runoff = 0.4 cfs @ 12.00 hrs, Volume= 1,242 cf, Depth= 4.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
3,116	98	Roofs, HSG C
3,116		100.00% Impervious Area

Summary for Subcatchment RF-05: RF-05

Runoff = 0.4 cfs @ 12.00 hrs, Volume= 1,242 cf, Depth= 4.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
3,116	98	Roofs, HSG C
3,116		100.00% Impervious Area

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 58

Summary for Subcatchment RF-06: RF-06

Runoff = 0.4 cfs @ 12.00 hrs, Volume= 1,242 cf, Depth= 4.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.02"

Area (sf)	CN	Description
3,116	98	Roofs, HSG C
3,116		100.00% Impervious Area

Summary for Pond 1P: Pond

Inflow Area = 259,181 sf, 16.20% Impervious, Inflow Depth = 1.36" for 10-year event
 Inflow = 5.8 cfs @ 12.32 hrs, Volume= 29,307 cf
 Outflow = 5.8 cfs @ 12.33 hrs, Volume= 29,307 cf, Atten= 0%, Lag= 0.3 min
 Primary = 5.8 cfs @ 12.33 hrs, Volume= 29,307 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 320.37' @ 12.33 hrs Surf.Area= 481 sf Storage= 139 cf

Plug-Flow detention time= 0.6 min calculated for 29,304 cf (100% of inflow)
 Center-of-Mass det. time= 0.6 min (832.2 - 831.6)

Volume	Invert	Avail.Storage	Storage Description
#1	320.00'	1,892 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
320.00	269	0	0
321.00	841	555	555
322.00	1,833	1,337	1,892

Device	Routing	Invert	Outlet Devices
#1	Primary	320.20'	8.0" Round Culvert L= 15.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 320.20' / 320.00' S= 0.0133 '/ Cc= 0.900 n= 0.013, Flow Area= 0.35 sf
#2	Primary	320.00'	10.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

Primary OutFlow Max=5.8 cfs @ 12.33 hrs HW=320.37' TW=0.00' (Dynamic Tailwater)

1=Culvert (Inlet Controls 0.1 cfs @ 1.11 fps)

2=Broad-Crested Rectangular Weir(Weir Controls 5.7 cfs @ 1.54 fps)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 59

Summary for Pond INF-103: INF-103

Inflow Area = 30,846 sf, 55.87% Impervious, Inflow Depth = 2.04" for 10-year event
 Inflow = 1.5 cfs @ 12.23 hrs, Volume= 5,236 cf
 Outflow = 1.5 cfs @ 12.25 hrs, Volume= 5,236 cf, Atten= 1%, Lag= 1.3 min
 Discarded = 0.0 cfs @ 7.57 hrs, Volume= 313 cf
 Primary = 1.5 cfs @ 12.25 hrs, Volume= 4,923 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 326.96' @ 12.25 hrs Surf.Area= 473 sf Storage= 249 cf

Plug-Flow detention time= 30.7 min calculated for 5,235 cf (100% of inflow)
 Center-of-Mass det. time= 30.8 min (834.0 - 803.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	326.00'	310 cf	12.42'W x 38.07'L x 2.00'H Field A 945 cf Overall - 171 cf Embedded = 774 cf x 40.0% Voids
#2A	326.50'	171 cf	ADS_StormTech SC-160LP +Cap x 25 Inside #1 Effective Size= 18.0"W x 12.0"H => 0.96 sf x 7.12'L = 6.8 cf Overall Size= 25.0"W x 12.0"H x 7.56'L with 0.44' Overlap 25 Chambers in 5 Rows
		481 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	326.56'	6.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads
#2	Discarded	326.00'	0.276 in/hr Exfiltration - Loamy Sand over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.0 cfs @ 7.57 hrs HW=326.02' (Free Discharge)

↑**2=Exfiltration - Loamy Sand** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=1.5 cfs @ 12.25 hrs HW=326.96' TW=320.34' (Dynamic Tailwater)

↑**1=Orifice/Grate** (Orifice Controls 1.5 cfs @ 2.16 fps)

Summary for Pond INF-104: INF-104

Inflow Area = 156,047 sf, 9.19% Impervious, Inflow Depth = 1.56" for 10-year event
 Inflow = 2.8 cfs @ 12.62 hrs, Volume= 20,262 cf
 Outflow = 2.8 cfs @ 12.66 hrs, Volume= 20,262 cf, Atten= 2%, Lag= 2.5 min
 Discarded = 0.1 cfs @ 11.37 hrs, Volume= 7,727 cf
 Primary = 2.6 cfs @ 12.66 hrs, Volume= 12,534 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 327.32' @ 12.66 hrs Surf.Area= 1,889 sf Storage= 2,849 cf

Plug-Flow detention time= 73.4 min calculated for 20,260 cf (100% of inflow)
 Center-of-Mass det. time= 73.4 min (914.9 - 841.5)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 60

Volume	Invert	Avail.Storage	Storage Description
#1A	325.10'	1,726 cf	25.25'W x 74.82'L x 3.50'H Field A 6,612 cf Overall - 2,297 cf Embedded = 4,315 cf x 40.0% Voids
#2A	325.60'	2,297 cf	ADS_StormTech SC-740 +Cap x 50 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 50 Chambers in 5 Rows
		4,023 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	327.14'	6.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads
#2	Primary	326.81'	10.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	326.64'	12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	325.10'	3.072 in/hr Exfiltration - Loamy Sand over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.1 cfs @ 11.37 hrs HW=325.14' (Free Discharge)↑**4=Exfiltration - Loamy Sand** (Exfiltration Controls 0.1 cfs)**Primary OutFlow** Max=2.6 cfs @ 12.66 hrs HW=327.32' TW=320.30' (Dynamic Tailwater)↑**1=Orifice/Grate** (Orifice Controls 0.2 cfs @ 1.44 fps)|**2=Orifice/Grate** (Orifice Controls 0.8 cfs @ 2.43 fps)|**3=Orifice/Grate** (Orifice Controls 1.6 cfs @ 2.81 fps)**Summary for Pond INF-105: INF-105**

Inflow Area =	92,080 sf,	9.15% Impervious,	Inflow Depth = 2.30" for 10-year event
Inflow =	3.7 cfs @	12.32 hrs,	Volume= 17,619 cf
Outflow =	1.8 cfs @	12.68 hrs,	Volume= 17,619 cf, Atten= 51%, Lag= 22.0 min
Discarded =	0.1 cfs @	11.30 hrs,	Volume= 10,243 cf
Primary =	1.7 cfs @	12.68 hrs,	Volume= 7,376 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 348.79' @ 12.68 hrs Surf.Area= 3,941 sf Storage= 6,206 cf

Plug-Flow detention time= 282.5 min calculated for 17,617 cf (100% of inflow)

Center-of-Mass det. time= 282.5 min (1,136.9 - 854.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	346.50'	3,532 cf	44.25'W x 89.06'L x 3.50'H Field A 13,793 cf Overall - 4,962 cf Embedded = 8,831 cf x 40.0% Voids
#2A	347.00'	4,962 cf	ADS_StormTech SC-740 +Cap x 108 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 108 Chambers in 9 Rows
		8,494 cf	Total Available Storage

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 61

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	348.54'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	348.38'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	348.21'	10.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	346.50'	1.284 in/hr Exfiltration - Sandy Loam over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.1 cfs @ 11.30 hrs HW=346.54' (Free Discharge)

↳ **4=Exfiltration - Sandy Loam** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=1.7 cfs @ 12.68 hrs HW=348.79' TW=327.32' (Dynamic Tailwater)

↳ **1=Orifice/Grate** (Orifice Controls 0.2 cfs @ 1.69 fps)

↳ **2=Orifice/Grate** (Orifice Controls 0.5 cfs @ 2.17 fps)

↳ **3=Orifice/Grate** (Orifice Controls 1.0 cfs @ 2.59 fps)

Summary for Pond INF-106: INF-106

Inflow Area =	22,980 sf, 57.09% Impervious, Inflow Depth = 3.69" for 10-year event
Inflow =	2.1 cfs @ 12.10 hrs, Volume= 7,062 cf
Outflow =	1.1 cfs @ 12.25 hrs, Volume= 7,062 cf, Atten= 47%, Lag= 8.9 min
Discarded =	0.0 cfs @ 9.26 hrs, Volume= 4,178 cf
Primary =	1.1 cfs @ 12.25 hrs, Volume= 2,885 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 356.14' @ 12.25 hrs Surf.Area= 786 sf Storage= 2,590 cf

Plug-Flow detention time= 432.4 min calculated for 7,062 cf (100% of inflow)

Center-of-Mass det. time= 432.5 min (1,231.3 - 798.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	351.25'	1,389 cf	28.50'W x 27.59'L x 6.75'H Field A 5,308 cf Overall - 1,834 cf Embedded = 3,474 cf x 40.0% Voids
#2A	352.00'	1,834 cf	ADS_StormTech MC-4500 b +Cap x 15 Inside #1 Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 15 Chambers in 3 Rows Cap Storage= +39.5 cf x 2 x 3 rows = 237.0 cf
		3,224 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	355.55'	6.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads
#2	Discarded	351.25'	1.758 in/hr Exfiltration - Sandy Loam over Surface area Phase-In= 0.01'

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 62

Discarded OutFlow Max=0.0 cfs @ 9.26 hrs HW=351.32' (Free Discharge)↑**2=Exfiltration - Sandy Loam** (Exfiltration Controls 0.0 cfs)**Primary OutFlow** Max=1.1 cfs @ 12.25 hrs HW=356.14' TW=326.96' (Dynamic Tailwater)↑**1=Orifice/Grate** (Orifice Controls 1.1 cfs @ 2.83 fps)**Summary for Pond INF-201: INF-201**

Inflow Area = 129,607 sf, 4.54% Impervious, Inflow Depth = 2.22" for 10-year event
 Inflow = 4.6 cfs @ 12.31 hrs, Volume= 23,983 cf
 Outflow = 3.9 cfs @ 12.45 hrs, Volume= 23,983 cf, Atten= 15%, Lag= 8.5 min
 Discarded = 0.2 cfs @ 11.24 hrs, Volume= 10,466 cf
 Primary = 3.7 cfs @ 12.45 hrs, Volume= 13,517 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 338.46' @ 12.45 hrs Surf.Area= 3,236 sf Storage= 5,013 cf

Plug-Flow detention time= 99.5 min calculated for 23,983 cf (100% of inflow)

Center-of-Mass det. time= 99.5 min (951.2 - 851.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	336.20'	2,914 cf	39.50'W x 81.94'L x 3.50'H Field A 11,328 cf Overall - 4,043 cf Embedded = 7,285 cf x 40.0% Voids
#2A	336.70'	4,043 cf	ADS_StormTech SC-740 +Cap x 88 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 88 Chambers in 8 Rows
		6,957 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	337.45'	15.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	338.24'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Discarded	336.20'	2.262 in/hr Exfiltration - Sandy Loam over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.2 cfs @ 11.24 hrs HW=336.24' (Free Discharge)↑**3=Exfiltration - Sandy Loam** (Exfiltration Controls 0.2 cfs)**Primary OutFlow** Max=3.7 cfs @ 12.45 hrs HW=338.46' TW=0.00' (Dynamic Tailwater)↑**1=Orifice/Grate** (Orifice Controls 3.6 cfs @ 3.41 fps)↑**2=Orifice/Grate** (Orifice Controls 0.1 cfs @ 1.58 fps)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 63

Summary for Pond INF-RF2: INF-RF2

Inflow Area = 3,116 sf, 100.00% Impervious, Inflow Depth = 4.78" for 10-year event
 Inflow = 0.4 cfs @ 12.00 hrs, Volume= 1,242 cf
 Outflow = 0.4 cfs @ 12.01 hrs, Volume= 1,242 cf, Atten= 4%, Lag= 0.6 min
 Discarded = 0.0 cfs @ 7.50 hrs, Volume= 518 cf
 Primary = 0.4 cfs @ 12.01 hrs, Volume= 724 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 335.80' @ 12.01 hrs Surf.Area= 118 sf Storage= 123 cf

Plug-Flow detention time= 61.7 min calculated for 1,242 cf (100% of inflow)
 Center-of-Mass det. time= 61.7 min (804.1 - 742.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	334.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	334.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	334.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	335.38'	8.0" Round Culvert L= 52.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.38' / 333.00' S= 0.0458 '/' Cc= 0.900 n= 0.013, Flow Area= 0.35 sf

Discarded OutFlow Max=0.0 cfs @ 7.50 hrs HW=334.04' (Free Discharge)
 ↑1=Exfiltration (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=0.4 cfs @ 12.01 hrs HW=335.80' TW=0.00' (Dynamic Tailwater)
 ↑2=Culvert (Inlet Controls 0.4 cfs @ 1.74 fps)

Summary for Pond INF-RF3: INF-RF3

Inflow Area = 1,685 sf, 100.00% Impervious, Inflow Depth = 4.78" for 10-year event
 Inflow = 0.2 cfs @ 12.00 hrs, Volume= 672 cf
 Outflow = 0.2 cfs @ 12.01 hrs, Volume= 672 cf, Atten= 6%, Lag= 0.8 min
 Discarded = 0.0 cfs @ 9.38 hrs, Volume= 411 cf
 Primary = 0.2 cfs @ 12.01 hrs, Volume= 260 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 333.67' @ 12.01 hrs Surf.Area= 118 sf Storage= 113 cf

Plug-Flow detention time= 72.3 min calculated for 672 cf (100% of inflow)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 64

Center-of-Mass det. time= 72.3 min (814.7 - 742.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	332.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	332.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	332.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	333.38'	8.0" Round Culvert L= 57.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 333.38' / 331.00' S= 0.0418 '/ Cc= 0.900 n= 0.013, Flow Area= 0.35 sf

Discarded OutFlow Max=0.0 cfs @ 9.38 hrs HW=332.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.0 cfs)**Primary OutFlow** Max=0.2 cfs @ 12.01 hrs HW=333.67' TW=0.00' (Dynamic Tailwater)↑**2=Culvert** (Inlet Controls 0.2 cfs @ 1.45 fps)**Summary for Pond INF-RF4: INF-RF4**

Volume	Invert	Avail.Storage	Storage Description
#1A	366.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	366.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	366.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	367.38'	8.0" Round Culvert L= 37.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 367.38' / 366.00' S= 0.0373 '/ Cc= 0.900 n= 0.013, Flow Area= 0.35 sf

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 65

Discarded OutFlow Max=0.0 cfs @ 0.00 hrs HW=0.00' (Free Discharge)

↑**1=Exfiltration** (Controls 0.0 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=0.00' (Free Discharge)

↑**2=Culvert** (Controls 0.0 cfs)

Summary for Pond INF-RF5: INF-RF5

Inflow Area = 3,116 sf, 100.00% Impervious, Inflow Depth = 4.78" for 10-year event
 Inflow = 0.4 cfs @ 12.00 hrs, Volume= 1,242 cf
 Outflow = 0.4 cfs @ 12.01 hrs, Volume= 1,242 cf, Atten= 4%, Lag= 0.6 min
 Discarded = 0.0 cfs @ 7.50 hrs, Volume= 518 cf
 Primary = 0.4 cfs @ 12.01 hrs, Volume= 724 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 377.80' @ 12.01 hrs Surf.Area= 118 sf Storage= 123 cf

Plug-Flow detention time= 61.7 min calculated for 1,242 cf (100% of inflow)
 Center-of-Mass det. time= 61.7 min (804.1 - 742.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	376.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	376.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	377.38'	8.0" Round Culvert L= 16.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 377.38' / 376.00' S= 0.0862 '/' Cc= 0.900 n= 0.013, Flow Area= 0.35 sf
#2	Discarded	376.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.0 cfs @ 7.50 hrs HW=376.04' (Free Discharge)

↑**2=Exfiltration** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=0.4 cfs @ 12.01 hrs HW=377.80' TW=337.04' (Dynamic Tailwater)

↑**1=Culvert** (Inlet Controls 0.4 cfs @ 1.74 fps)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 10-year Rainfall=5.02"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 66

Summary for Pond INF-RF6: INF-RF6

Inflow Area = 3,116 sf, 100.00% Impervious, Inflow Depth = 4.78" for 10-year event
 Inflow = 0.4 cfs @ 12.00 hrs, Volume= 1,242 cf
 Outflow = 0.4 cfs @ 12.01 hrs, Volume= 1,242 cf, Atten= 4%, Lag= 0.6 min
 Discarded = 0.0 cfs @ 7.50 hrs, Volume= 518 cf
 Primary = 0.4 cfs @ 12.01 hrs, Volume= 724 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 377.80' @ 12.01 hrs Surf.Area= 118 sf Storage= 123 cf

Plug-Flow detention time= 61.7 min calculated for 1,242 cf (100% of inflow)
 Center-of-Mass det. time= 61.7 min (804.1 - 742.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	376.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	376.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	377.38'	8.0" Round Culvert L= 58.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 377.38' / 376.00' S= 0.0238 '/' Cc= 0.900 n= 0.013, Flow Area= 0.35 sf
#2	Discarded	376.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.0 cfs @ 7.50 hrs HW=376.04' (Free Discharge)
 ↑**2=Exfiltration** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=0.4 cfs @ 12.01 hrs HW=377.80' TW=0.00' (Dynamic Tailwater)
 ↑**1=Culvert** (Inlet Controls 0.4 cfs @ 1.74 fps)

Summary for Link DP-1: DESIGN POINT 1

Inflow Area = 303,248 sf, 14.83% Impervious, Inflow Depth = 1.22" for 10-year event
 Inflow = 5.9 cfs @ 12.33 hrs, Volume= 30,953 cf
 Primary = 5.9 cfs @ 12.33 hrs, Volume= 30,953 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Summary for Link DP-2: DESIGN POINT 2

Inflow Area = 451,037 sf, 7.83% Impervious, Inflow Depth = 1.94" for 10-year event
Inflow = 15.3 cfs @ 12.33 hrs, Volume= 73,050 cf
Primary = 15.3 cfs @ 12.33 hrs, Volume= 73,050 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Time span=0.00-96.00 hrs, dt=0.01 hrs, 9601 points x 2
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentPDA-100: PDA-100	Runoff Area=44,067 sf 6.77% Impervious Runoff Depth=1.62" Flow Length=263' Tc=15.1 min CN=45 Runoff=1.2 cfs 5,954 cf
SubcatchmentPDA-101: PDA-101	Runoff Area=8,231 sf 34.50% Impervious Runoff Depth=3.17" Flow Length=178' Tc=6.0 min CN=60 Runoff=0.7 cfs 2,174 cf
SubcatchmentPDA-102: PDA-102	Runoff Area=64,057 sf 11.83% Impervious Runoff Depth=4.28" Flow Length=618' Tc=15.8 min CN=70 Runoff=5.5 cfs 22,825 cf
SubcatchmentPDA-103: PDA-103	Runoff Area=7,866 sf 52.29% Impervious Runoff Depth=6.24" Flow Length=356' Tc=6.0 min CN=87 Runoff=1.3 cfs 4,087 cf
SubcatchmentPDA-104: PDA-104	Runoff Area=60,851 sf 4.61% Impervious Runoff Depth=4.62" Flow Length=889' Tc=17.5 min CN=73 Runoff=5.4 cfs 23,405 cf
SubcatchmentPDA-105: PDA-105	Runoff Area=92,080 sf 9.15% Impervious Runoff Depth=4.62" Flow Length=797' Tc=21.7 min CN=73 Runoff=7.4 cfs 35,416 cf
SubcatchmentPDA-106: PDA-106	Runoff Area=22,980 sf 57.09% Impervious Runoff Depth=6.35" Flow Length=489' Slope=0.0460 '/' Tc=7.2 min CN=88 Runoff=3.6 cfs 12,166 cf
SubcatchmentPDA-200: PDA-200	Runoff Area=313,513 sf 6.86% Impervious Runoff Depth=4.50" Flow Length=966' Tc=20.0 min CN=72 Runoff=25.6 cfs 117,618 cf
SubcatchmentPDA-201: Subcat PDA-201	Runoff Area=15,861 sf 17.40% Impervious Runoff Depth=5.19" Flow Length=228' Tc=6.0 min CN=78 Runoff=2.2 cfs 6,857 cf
SubcatchmentPDA-202: Subcat PDA-202	Runoff Area=110,630 sf 0.01% Impervious Runoff Depth=4.39" Flow Length=830' Tc=22.6 min CN=71 Runoff=8.4 cfs 40,460 cf
SubcatchmentRF-02: RF-02	Runoff Area=3,116 sf 100.00% Impervious Runoff Depth=7.54" Tc=0.0 min CN=98 Runoff=0.7 cfs 1,958 cf
SubcatchmentRF-03: RF-03	Runoff Area=1,685 sf 100.00% Impervious Runoff Depth=7.54" Tc=0.0 min CN=98 Runoff=0.4 cfs 1,059 cf
SubcatchmentRF-04: RF-04	Runoff Area=3,116 sf 100.00% Impervious Runoff Depth=7.54" Tc=0.0 min CN=98 Runoff=0.7 cfs 1,958 cf
SubcatchmentRF-05: RF-05	Runoff Area=3,116 sf 100.00% Impervious Runoff Depth=7.54" Tc=0.0 min CN=98 Runoff=0.7 cfs 1,958 cf
SubcatchmentRF-06: RF-06	Runoff Area=3,116 sf 100.00% Impervious Runoff Depth=7.54" Tc=0.0 min CN=98 Runoff=0.7 cfs 1,958 cf
Pond 1P: Pond	Peak Elev=320.71' Storage=338 cf Inflow=16.8 cfs 76,154 cf Outflow=16.8 cfs 76,154 cf

Pond INF-103: INF-103 Peak Elev=327.76' Storage=434 cf Inflow=3.7 cfs 11,784 cf
 Discarded=0.0 cfs 335 cf Primary=3.7 cfs 11,450 cf Outflow=3.7 cfs 11,784 cf

Pond INF-104: INF-104 Peak Elev=328.59' Storage=4,012 cf Inflow=10.1 cfs 49,352 cf
 Discarded=0.1 cfs 9,646 cf Primary=9.7 cfs 39,706 cf Outflow=9.8 cfs 49,352 cf

Pond INF-105: INF-105 Peak Elev=349.93' Storage=8,379 cf Inflow=7.4 cfs 35,416 cf
 Discarded=0.1 cfs 11,427 cf Primary=5.9 cfs 23,989 cf Outflow=6.0 cfs 35,416 cf

Pond INF-106: INF-106 Peak Elev=357.92' Storage=3,199 cf Inflow=3.6 cfs 12,166 cf
 Discarded=0.0 cfs 4,470 cf Primary=2.8 cfs 7,697 cf Outflow=2.8 cfs 12,166 cf

Pond INF-201: INF-201 Peak Elev=339.67' Storage=6,918 cf Inflow=9.4 cfs 48,674 cf
 Discarded=0.2 cfs 12,274 cf Primary=8.5 cfs 36,400 cf Outflow=8.7 cfs 48,674 cf

Pond INF-RF2: INF-RF2 Peak Elev=335.94' Storage=133 cf Inflow=0.7 cfs 1,958 cf
 Discarded=0.0 cfs 601 cf Primary=0.6 cfs 1,357 cf Outflow=0.6 cfs 1,958 cf

Pond INF-RF3: INF-RF3 Peak Elev=333.76' Storage=120 cf Inflow=0.4 cfs 1,059 cf
 Discarded=0.0 cfs 499 cf Primary=0.3 cfs 560 cf Outflow=0.3 cfs 1,059 cf

Pond INF-RF4: INF-RF4 Peak Elev=0.00' Storage=0 cf
 Discarded=0.0 cfs 0 cf Primary=0.0 cfs 0 cf

Pond INF-RF5: INF-RF5 Peak Elev=377.94' Storage=133 cf Inflow=0.7 cfs 1,958 cf
 Discarded=0.0 cfs 601 cf Primary=0.6 cfs 1,357 cf Outflow=0.6 cfs 1,958 cf

Pond INF-RF6: INF-RF6 Peak Elev=377.94' Storage=133 cf Inflow=0.7 cfs 1,958 cf
 Discarded=0.0 cfs 601 cf Primary=0.6 cfs 1,357 cf Outflow=0.6 cfs 1,958 cf

Link DP-1: DESIGN POINT 1 Inflow=18.0 cfs 82,108 cf
 Primary=18.0 cfs 82,108 cf

Link DP-2: DESIGN POINT 2 Inflow=33.9 cfs 157,291 cf
 Primary=33.9 cfs 157,291 cf

Total Runoff Area = 754,285 sf Runoff Volume = 279,854 cf Average Runoff Depth = 4.45"
89.36% Pervious = 673,994 sf 10.64% Impervious = 80,291 sf

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 70

Summary for Subcatchment PDA-100: PDA-100

Runoff = 1.2 cfs @ 12.24 hrs, Volume= 5,954 cf, Depth= 1.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
2,894	98	Paved parking, HSG A
37,895	39	>75% Grass cover, Good, HSG A
395	70	Woods, Good, HSG C
2,795	74	>75% Grass cover, Good, HSG C
88	98	Paved parking, HSG D
44,067	45	Weighted Average
41,085		93.23% Pervious Area
2,982		6.77% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.2	50	0.0256	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
3.9	213	0.0170	0.91		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
15.1	263	Total			

Summary for Subcatchment PDA-101: PDA-101

Runoff = 0.7 cfs @ 12.09 hrs, Volume= 2,174 cf, Depth= 3.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
5,156	39	>75% Grass cover, Good, HSG A
2,334	98	Paved parking, HSG A
235	74	>75% Grass cover, Good, HSG C
506	98	Paved parking, HSG C
8,231	60	Weighted Average
5,391		65.50% Pervious Area
2,840		34.50% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.4	50	0.0900	2.19		Sheet Flow, Smooth surfaces n= 0.011 P2= 3.20"
0.4	128	0.1076	5.28		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
0.8	178	Total, Increased to minimum Tc = 6.0 min			

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 71

Summary for Subcatchment PDA-102: PDA-102

Runoff = 5.5 cfs @ 12.22 hrs, Volume= 22,825 cf, Depth= 4.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
11,117	39	>75% Grass cover, Good, HSG A
3,051	98	Paved parking, HSG A
19,777	70	Woods, Good, HSG C
25,414	74	>75% Grass cover, Good, HSG C
2,188	98	Paved parking, HSG C
2,272	98	Roofs, HSG C
173	80	>75% Grass cover, Good, HSG D
66	98	Paved parking, HSG D
64,057	70	Weighted Average
56,481		88.17% Pervious Area
7,576		11.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.1	50	0.0908	0.27		Sheet Flow, Grass: Short n= 0.150 P2= 3.20"
8.5	360	0.0200	0.71		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
4.2	208	0.0240	0.82	4.10	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00' n= 0.200
15.8	618	Total			

Summary for Subcatchment PDA-103: PDA-103

Runoff = 1.3 cfs @ 12.08 hrs, Volume= 4,087 cf, Depth= 6.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
3,753	74	>75% Grass cover, Good, HSG C
4,113	98	Paved parking, HSG C
0	70	Woods, Good, HSG C
7,866	87	Weighted Average
3,753		47.71% Pervious Area
4,113		52.29% Impervious Area

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 72

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.3	36	0.0611	1.75		Sheet Flow, Smooth surfaces n= 0.011 P2= 3.20"
0.4	188	0.0189	8.17	14.44	Pipe Channel, 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013
0.2	132	0.0902	10.59	52.95	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 '/' Top.W=8.00' n= 0.030 Short grass
0.9	356	Total, Increased to minimum Tc = 6.0 min			

Summary for Subcatchment PDA-104: PDA-104

Runoff = 5.4 cfs @ 12.23 hrs, Volume= 23,405 cf, Depth= 4.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
19,407	74	>75% Grass cover, Good, HSG C
38,640	70	Woods, Good, HSG C
2,804	98	Paved parking, HSG C
60,851	73	Weighted Average
58,047		95.39% Pervious Area
2,804		4.61% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.2	50	0.0324	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
4.3	335	0.0689	1.31		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
1.1	151	0.1167	2.39		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1.6	131	0.0730	1.35		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.3	222	0.1039	11.16	3.90	Pipe Channel, 8.0" Round Area= 0.3 sf Perim= 2.1' r= 0.17' n= 0.013
17.5	889	Total			

Summary for Subcatchment PDA-105: PDA-105

Runoff = 7.4 cfs @ 12.30 hrs, Volume= 35,416 cf, Depth= 4.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 73

Area (sf)	CN	Description
65,985	70	Woods, Good, HSG C
8,422	98	Paved parking, HSG C
17,673	74	>75% Grass cover, Good, HSG C
92,080	73	Weighted Average
83,658		90.85% Pervious Area
8,422		9.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.0	50	0.0176	0.06		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
5.0	347	0.0528	1.15		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.1	30	0.3300	4.02		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.8	208	0.0684	4.21		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
2.8	162	0.0333	0.97	4.83	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 ' Top.W=8.00' n= 0.200
21.7	797	Total			

Summary for Subcatchment PDA-106: PDA-106

Runoff = 3.6 cfs @ 12.10 hrs, Volume= 12,166 cf, Depth= 6.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
9,097	74	>75% Grass cover, Good, HSG C
13,120	98	Paved parking, HSG C
763	70	Woods, Good, HSG C
22,980	88	Weighted Average
9,860		42.91% Pervious Area
13,120		57.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.2	489	0.0460	1.13	5.67	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 ' Top.W=8.00' n= 0.200

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 74

Summary for Subcatchment PDA-200: PDA-200

Runoff = 25.6 cfs @ 12.28 hrs, Volume= 117,618 cf, Depth= 4.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
25,377	55	Woods, Good, HSG B
36,942	74	>75% Grass cover, Good, HSG C
191,740	70	Woods, Good, HSG C
3,065	39	>75% Grass cover, Good, HSG A
295	98	Roofs, HSG C
3,073	98	Paved parking, HSG C
277	98	Paved parking, HSG A
266	98	Paved parking, HSG D
11,813	98	Water Surface, HSG C
5,791	98	Water Surface, HSG D
25,648	77	Woods, Good, HSG D
9,226	80	>75% Grass cover, Good, HSG D
313,513	72	Weighted Average
291,998		93.14% Pervious Area
21,515		6.86% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	50	0.0296	0.08		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
3.4	247	0.0587	1.21		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
2.6	248	0.0524	1.60		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1.8	207	0.1498	1.94		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.4	87	0.0274	3.36		Shallow Concentrated Flow, Paved Kv= 20.3 fps
0.1	16	0.3333	4.04		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
1.2	111	0.0901	1.50		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
20.0	966	Total			

Summary for Subcatchment PDA-201: Subcat PDA-201

Runoff = 2.2 cfs @ 12.09 hrs, Volume= 6,857 cf, Depth= 5.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

85 West Street Northborough JN-1207.03 - Proposed

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 75

Area (sf)	CN	Description
12,163	74	>75% Grass cover, Good, HSG C
938	70	Woods, Good, HSG C
2,760	98	Paved parking, HSG C
15,861	78	Weighted Average
13,101		82.60% Pervious Area
2,760		17.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	31	0.0200	1.09		Sheet Flow, Smooth surfaces n= 0.011 P2= 3.20"
0.3	197	0.0609	13.05	65.26	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 ' /' Top.W=8.00' n= 0.020
0.8	228	Total, Increased to minimum Tc = 6.0 min			

Summary for Subcatchment PDA-202: Subcat PDA-202

Runoff = 8.4 cfs @ 12.31 hrs, Volume= 40,460 cf, Depth= 4.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
41,066	74	>75% Grass cover, Good, HSG C
10	98	Paved parking, HSG C
69,554	70	Woods, Good, HSG C
110,630	71	Weighted Average
110,620		99.99% Pervious Area
10		0.01% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.3	50	0.0250	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.20"
9.2	308	0.0500	0.56		Shallow Concentrated Flow, Forest w/Heavy Litter Kv= 2.5 fps
1.7	169	0.0592	1.70		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.4	303	0.0460	11.34	56.72	Trap/Vee/Rect Channel Flow, Bot.W=2.00' D=1.00' Z= 3.0 ' /' Top.W=8.00' n= 0.020
22.6	830	Total			

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 76

Summary for Subcatchment RF-02: RF-02

Runoff = 0.7 cfs @ 12.00 hrs, Volume= 1,958 cf, Depth= 7.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
3,116	98	Roofs, HSG C
3,116		100.00% Impervious Area

Summary for Subcatchment RF-03: RF-03

Runoff = 0.4 cfs @ 12.00 hrs, Volume= 1,059 cf, Depth= 7.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
1,685	98	Roofs, HSG C
1,685		100.00% Impervious Area

Summary for Subcatchment RF-04: RF-04

Runoff = 0.7 cfs @ 12.00 hrs, Volume= 1,958 cf, Depth= 7.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
3,116	98	Roofs, HSG C
3,116		100.00% Impervious Area

Summary for Subcatchment RF-05: RF-05

Runoff = 0.7 cfs @ 12.00 hrs, Volume= 1,958 cf, Depth= 7.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
3,116	98	Roofs, HSG C
3,116		100.00% Impervious Area

Summary for Subcatchment RF-06: RF-06

Runoff = 0.7 cfs @ 12.00 hrs, Volume= 1,958 cf, Depth= 7.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs
 Type III 24-hr 100-year Rainfall=7.78"

Area (sf)	CN	Description
3,116	98	Roofs, HSG C
3,116		100.00% Impervious Area

Summary for Pond 1P: Pond

Inflow Area = 259,181 sf, 16.20% Impervious, Inflow Depth = 3.53" for 100-year event
 Inflow = 16.8 cfs @ 12.30 hrs, Volume= 76,154 cf
 Outflow = 16.8 cfs @ 12.30 hrs, Volume= 76,154 cf, Atten= 0%, Lag= 0.3 min
 Primary = 16.8 cfs @ 12.30 hrs, Volume= 76,154 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 320.71' @ 12.30 hrs Surf.Area= 678 sf Storage= 338 cf

Plug-Flow detention time= 0.7 min calculated for 76,154 cf (100% of inflow)
 Center-of-Mass det. time= 0.5 min (821.0 - 820.5)

Volume	Invert	Avail.Storage	Storage Description
#1	320.00'	1,892 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
320.00	269	0	0
321.00	841	555	555
322.00	1,833	1,337	1,892

Device	Routing	Invert	Outlet Devices
#1	Primary	320.20'	8.0" Round Culvert L= 15.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 320.20' / 320.00' S= 0.0133 '/ Cc= 0.900 n= 0.013, Flow Area= 0.35 sf
#2	Primary	320.00'	10.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74

Primary OutFlow Max=16.8 cfs @ 12.30 hrs HW=320.71' TW=0.00' (Dynamic Tailwater)

- 1=Culvert (Inlet Controls 0.6 cfs @ 1.93 fps)
- 2=Broad-Crested Rectangular Weir (Weir Controls 16.3 cfs @ 2.28 fps)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 78

Summary for Pond INF-103: INF-103

Inflow Area = 30,846 sf, 55.87% Impervious, Inflow Depth = 4.58" for 100-year event
 Inflow = 3.7 cfs @ 12.13 hrs, Volume= 11,784 cf
 Outflow = 3.7 cfs @ 12.15 hrs, Volume= 11,784 cf, Atten= 2%, Lag= 1.7 min
 Discarded = 0.0 cfs @ 5.59 hrs, Volume= 335 cf
 Primary = 3.7 cfs @ 12.15 hrs, Volume= 11,450 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 327.76' @ 12.15 hrs Surf.Area= 473 sf Storage= 434 cf

Plug-Flow detention time= 15.9 min calculated for 11,783 cf (100% of inflow)
 Center-of-Mass det. time= 16.0 min (813.1 - 797.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	326.00'	310 cf	12.42'W x 38.07'L x 2.00'H Field A 945 cf Overall - 171 cf Embedded = 774 cf x 40.0% Voids
#2A	326.50'	171 cf	ADS_StormTech SC-160LP +Cap x 25 Inside #1 Effective Size= 18.0"W x 12.0"H => 0.96 sf x 7.12'L = 6.8 cf Overall Size= 25.0"W x 12.0"H x 7.56'L with 0.44' Overlap 25 Chambers in 5 Rows
		481 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	326.56'	6.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads
#2	Discarded	326.00'	0.276 in/hr Exfiltration - Loamy Sand over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.0 cfs @ 5.59 hrs HW=326.02' (Free Discharge)

↑**2=Exfiltration - Loamy Sand** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=3.7 cfs @ 12.15 hrs HW=327.75' TW=320.61' (Dynamic Tailwater)

↑**1=Orifice/Grate** (Orifice Controls 3.7 cfs @ 4.68 fps)

Summary for Pond INF-104: INF-104

Inflow Area = 156,047 sf, 9.19% Impervious, Inflow Depth = 3.80" for 100-year event
 Inflow = 10.1 cfs @ 12.35 hrs, Volume= 49,352 cf
 Outflow = 9.8 cfs @ 12.41 hrs, Volume= 49,352 cf, Atten= 2%, Lag= 4.1 min
 Discarded = 0.1 cfs @ 10.04 hrs, Volume= 9,646 cf
 Primary = 9.7 cfs @ 12.41 hrs, Volume= 39,706 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 328.59' @ 12.41 hrs Surf.Area= 1,889 sf Storage= 4,012 cf

Plug-Flow detention time= 41.6 min calculated for 49,347 cf (100% of inflow)
 Center-of-Mass det. time= 41.6 min (871.2 - 829.6)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 79

Volume	Invert	Avail.Storage	Storage Description
#1A	325.10'	1,726 cf	25.25'W x 74.82'L x 3.50'H Field A 6,612 cf Overall - 2,297 cf Embedded = 4,315 cf x 40.0% Voids
#2A	325.60'	2,297 cf	ADS_StormTech SC-740 +Cap x 50 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 50 Chambers in 5 Rows
		4,023 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	327.14'	6.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads
#2	Primary	326.81'	10.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	326.64'	12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	325.10'	3.072 in/hr Exfiltration - Loamy Sand over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.1 cfs @ 10.04 hrs HW=325.14' (Free Discharge)

↳ **4=Exfiltration - Loamy Sand** (Exfiltration Controls 0.1 cfs)

Primary OutFlow Max=9.7 cfs @ 12.41 hrs HW=328.59' TW=320.68' (Dynamic Tailwater)

↳ **1=Orifice/Grate** (Orifice Controls 2.1 cfs @ 5.26 fps)

↳ **2=Orifice/Grate** (Orifice Controls 3.1 cfs @ 5.61 fps)

↳ **3=Orifice/Grate** (Orifice Controls 4.5 cfs @ 5.79 fps)

Summary for Pond INF-105: INF-105

Inflow Area =	92,080 sf,	9.15% Impervious,	Inflow Depth = 4.62" for 100-year event
Inflow =	7.4 cfs @ 12.30 hrs,	Volume=	35,416 cf
Outflow =	6.0 cfs @ 12.46 hrs,	Volume=	35,416 cf, Atten= 20%, Lag= 9.7 min
Discarded =	0.1 cfs @ 9.80 hrs,	Volume=	11,427 cf
Primary =	5.9 cfs @ 12.46 hrs,	Volume=	23,989 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 349.93' @ 12.46 hrs Surf.Area= 3,941 sf Storage= 8,379 cf

Plug-Flow detention time= 162.5 min calculated for 35,413 cf (100% of inflow)

Center-of-Mass det. time= 162.5 min (996.7 - 834.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	346.50'	3,532 cf	44.25'W x 89.06'L x 3.50'H Field A 13,793 cf Overall - 4,962 cf Embedded = 8,831 cf x 40.0% Voids
#2A	347.00'	4,962 cf	ADS_StormTech SC-740 +Cap x 108 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 108 Chambers in 9 Rows
		8,494 cf	Total Available Storage

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 80

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	348.54'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	348.38'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	348.21'	10.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	346.50'	1.284 in/hr Exfiltration - Sandy Loam over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.1 cfs @ 9.80 hrs HW=346.54' (Free Discharge)↑**4=Exfiltration - Sandy Loam** (Exfiltration Controls 0.1 cfs)**Primary OutFlow** Max=5.9 cfs @ 12.46 hrs HW=349.93' TW=328.56' (Dynamic Tailwater)↑**1=Orifice/Grate** (Orifice Controls 1.0 cfs @ 5.13 fps)|**2=Orifice/Grate** (Orifice Controls 1.9 cfs @ 5.30 fps)|**3=Orifice/Grate** (Orifice Controls 3.0 cfs @ 5.49 fps)**Summary for Pond INF-106: INF-106**

Inflow Area =	22,980 sf, 57.09% Impervious, Inflow Depth = 6.35" for 100-year event
Inflow =	3.6 cfs @ 12.10 hrs, Volume= 12,166 cf
Outflow =	2.8 cfs @ 12.17 hrs, Volume= 12,166 cf, Atten= 22%, Lag= 4.2 min
Discarded =	0.0 cfs @ 7.59 hrs, Volume= 4,470 cf
Primary =	2.8 cfs @ 12.17 hrs, Volume= 7,697 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 357.92' @ 12.17 hrs Surf.Area= 786 sf Storage= 3,199 cf

Plug-Flow detention time= 271.0 min calculated for 12,165 cf (100% of inflow)

Center-of-Mass det. time= 271.1 min (1,055.1 - 784.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	351.25'	1,389 cf	28.50'W x 27.59'L x 6.75'H Field A 5,308 cf Overall - 1,834 cf Embedded = 3,474 cf x 40.0% Voids
#2A	352.00'	1,834 cf	ADS_StormTech MC-4500 b +Cap x 15 Inside #1 Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 15 Chambers in 3 Rows Cap Storage= +39.5 cf x 2 x 3 rows = 237.0 cf
		3,224 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	355.55'	6.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads
#2	Discarded	351.25'	1.758 in/hr Exfiltration - Sandy Loam over Surface area Phase-In= 0.01'

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 81

Discarded OutFlow Max=0.0 cfs @ 7.59 hrs HW=351.32' (Free Discharge)

↳ **2=Exfiltration - Sandy Loam** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=2.8 cfs @ 12.17 hrs HW=357.92' TW=327.75' (Dynamic Tailwater)

↳ **1=Orifice/Grate** (Orifice Controls 2.8 cfs @ 7.01 fps)

Summary for Pond INF-201: INF-201

Inflow Area = 129,607 sf, 4.54% Impervious, Inflow Depth = 4.51" for 100-year event
 Inflow = 9.4 cfs @ 12.30 hrs, Volume= 48,674 cf
 Outflow = 8.7 cfs @ 12.40 hrs, Volume= 48,674 cf, Atten= 8%, Lag= 6.0 min
 Discarded = 0.2 cfs @ 9.76 hrs, Volume= 12,274 cf
 Primary = 8.5 cfs @ 12.40 hrs, Volume= 36,400 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2

Peak Elev= 339.67' @ 12.40 hrs Surf.Area= 3,236 sf Storage= 6,918 cf

Plug-Flow detention time= 63.3 min calculated for 48,669 cf (100% of inflow)

Center-of-Mass det. time= 63.3 min (895.6 - 832.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	336.20'	2,914 cf	39.50"W x 81.94"L x 3.50"H Field A 11,328 cf Overall - 4,043 cf Embedded = 7,285 cf x 40.0% Voids
#2A	336.70'	4,043 cf	ADS_StormTech SC-740 +Cap x 88 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 88 Chambers in 8 Rows
		6,957 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	337.45'	15.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#2	Primary	338.24'	6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Discarded	336.20'	2.262 in/hr Exfiltration - Sandy Loam over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.2 cfs @ 9.76 hrs HW=336.24' (Free Discharge)

↳ **3=Exfiltration - Sandy Loam** (Exfiltration Controls 0.2 cfs)

Primary OutFlow Max=8.5 cfs @ 12.40 hrs HW=339.67' TW=0.00' (Dynamic Tailwater)

↳ **1=Orifice/Grate** (Orifice Controls 7.5 cfs @ 6.08 fps)

↳ **2=Orifice/Grate** (Orifice Controls 1.0 cfs @ 5.23 fps)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 82

Summary for Pond INF-RF2: INF-RF2

Inflow Area = 3,116 sf, 100.00% Impervious, Inflow Depth = 7.54" for 100-year event
 Inflow = 0.7 cfs @ 12.00 hrs, Volume= 1,958 cf
 Outflow = 0.6 cfs @ 12.01 hrs, Volume= 1,958 cf, Atten= 4%, Lag= 0.6 min
 Discarded = 0.0 cfs @ 5.21 hrs, Volume= 601 cf
 Primary = 0.6 cfs @ 12.01 hrs, Volume= 1,357 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 335.94' @ 12.01 hrs Surf.Area= 118 sf Storage= 133 cf

Plug-Flow detention time= 51.4 min calculated for 1,958 cf (100% of inflow)
 Center-of-Mass det. time= 51.4 min (787.4 - 736.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	334.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	334.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	334.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	335.38'	8.0" Round Culvert L= 52.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 335.38' / 333.00' S= 0.0458 '/' Cc= 0.900 n= 0.013, Flow Area= 0.35 sf

Discarded OutFlow Max=0.0 cfs @ 5.21 hrs HW=334.04' (Free Discharge)
 ↑1=Exfiltration (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=0.6 cfs @ 12.01 hrs HW=335.94' TW=0.00' (Dynamic Tailwater)
 ↑2=Culvert (Inlet Controls 0.6 cfs @ 2.01 fps)

Summary for Pond INF-RF3: INF-RF3

Inflow Area = 1,685 sf, 100.00% Impervious, Inflow Depth = 7.54" for 100-year event
 Inflow = 0.4 cfs @ 12.00 hrs, Volume= 1,059 cf
 Outflow = 0.3 cfs @ 12.01 hrs, Volume= 1,059 cf, Atten= 5%, Lag= 0.7 min
 Discarded = 0.0 cfs @ 8.01 hrs, Volume= 499 cf
 Primary = 0.3 cfs @ 12.01 hrs, Volume= 560 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 333.76' @ 12.01 hrs Surf.Area= 118 sf Storage= 120 cf

Plug-Flow detention time= 64.2 min calculated for 1,059 cf (100% of inflow)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 83

Center-of-Mass det. time= 64.2 min (800.2 - 736.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	332.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	332.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	332.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	333.38'	8.0" Round Culvert L= 57.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 333.38' / 331.00' S= 0.0418 '/ Cc= 0.900 n= 0.013, Flow Area= 0.35 sf

Discarded OutFlow Max=0.0 cfs @ 8.01 hrs HW=332.04' (Free Discharge)↑**1=Exfiltration** (Exfiltration Controls 0.0 cfs)**Primary OutFlow** Max=0.3 cfs @ 12.01 hrs HW=333.76' TW=0.00' (Dynamic Tailwater)↑**2=Culvert** (Inlet Controls 0.3 cfs @ 1.65 fps)**Summary for Pond INF-RF4: INF-RF4**

Volume	Invert	Avail.Storage	Storage Description
#1A	366.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	366.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	366.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	367.38'	8.0" Round Culvert L= 37.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 367.38' / 366.00' S= 0.0373 '/ Cc= 0.900 n= 0.013, Flow Area= 0.35 sf

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 84

Discarded OutFlow Max=0.0 cfs @ 0.00 hrs HW=0.00' (Free Discharge)

↑**1=Exfiltration** (Controls 0.0 cfs)

Primary OutFlow Max=0.0 cfs @ 0.00 hrs HW=0.00' (Free Discharge)

↑**2=Culvert** (Controls 0.0 cfs)

Summary for Pond INF-RF5: INF-RF5

Inflow Area = 3,116 sf, 100.00% Impervious, Inflow Depth = 7.54" for 100-year event
 Inflow = 0.7 cfs @ 12.00 hrs, Volume= 1,958 cf
 Outflow = 0.6 cfs @ 12.01 hrs, Volume= 1,958 cf, Atten= 4%, Lag= 0.6 min
 Discarded = 0.0 cfs @ 5.21 hrs, Volume= 601 cf
 Primary = 0.6 cfs @ 12.01 hrs, Volume= 1,357 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 377.94' @ 12.01 hrs Surf.Area= 118 sf Storage= 133 cf

Plug-Flow detention time= 51.4 min calculated for 1,958 cf (100% of inflow)
 Center-of-Mass det. time= 51.4 min (787.4 - 736.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	376.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	376.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	377.38'	8.0" Round Culvert L= 16.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 377.38' / 376.00' S= 0.0862 '/' Cc= 0.900 n= 0.013, Flow Area= 0.35 sf
#2	Discarded	376.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.0 cfs @ 5.21 hrs HW=376.04' (Free Discharge)

↑**2=Exfiltration** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=0.6 cfs @ 12.01 hrs HW=377.94' TW=338.28' (Dynamic Tailwater)

↑**1=Culvert** (Inlet Controls 0.6 cfs @ 2.01 fps)

85 West Street Northborough JN-1207.03 - Proposed Type III 24-hr 100-year Rainfall=7.78"

Prepared by WDA Design Group

Printed 5/3/2021

HydroCAD® 10.10-5a s/n 11557 © 2020 HydroCAD Software Solutions LLC

Page 85

Summary for Pond INF-RF6: INF-RF6

Inflow Area = 3,116 sf, 100.00% Impervious, Inflow Depth = 7.54" for 100-year event
 Inflow = 0.7 cfs @ 12.00 hrs, Volume= 1,958 cf
 Outflow = 0.6 cfs @ 12.01 hrs, Volume= 1,958 cf, Atten= 4%, Lag= 0.6 min
 Discarded = 0.0 cfs @ 5.21 hrs, Volume= 601 cf
 Primary = 0.6 cfs @ 12.01 hrs, Volume= 1,357 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 377.94' @ 12.01 hrs Surf.Area= 118 sf Storage= 133 cf

Plug-Flow detention time= 51.4 min calculated for 1,958 cf (100% of inflow)
 Center-of-Mass det. time= 51.4 min (787.4 - 736.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	376.00'	129 cf	11.00'W x 10.74'L x 3.50'H Field A 413 cf Overall - 92 cf Embedded = 321 cf x 40.0% Voids
#2A	376.50'	92 cf	ADS_StormTech SC-740 +Cap x 2 Inside #1 Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap 2 Chambers in 2 Rows
		220 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	377.38'	8.0" Round Culvert L= 58.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 377.38' / 376.00' S= 0.0238 '/' Cc= 0.900 n= 0.013, Flow Area= 0.35 sf
#2	Discarded	376.00'	2.410 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.0 cfs @ 5.21 hrs HW=376.04' (Free Discharge)
 ↑**2=Exfiltration** (Exfiltration Controls 0.0 cfs)

Primary OutFlow Max=0.6 cfs @ 12.01 hrs HW=377.94' TW=0.00' (Dynamic Tailwater)
 ↑**1=Culvert** (Inlet Controls 0.6 cfs @ 2.01 fps)

Summary for Link DP-1: DESIGN POINT 1

Inflow Area = 303,248 sf, 14.83% Impervious, Inflow Depth = 3.25" for 100-year event
 Inflow = 18.0 cfs @ 12.30 hrs, Volume= 82,108 cf
 Primary = 18.0 cfs @ 12.30 hrs, Volume= 82,108 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

Summary for Link DP-2: DESIGN POINT 2

Inflow Area = 451,037 sf, 7.83% Impervious, Inflow Depth = 4.18" for 100-year event
Inflow = 33.9 cfs @ 12.29 hrs, Volume= 157,291 cf
Primary = 33.9 cfs @ 12.29 hrs, Volume= 157,291 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-96.00 hrs, dt= 0.01 hrs

APPENDICES

Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

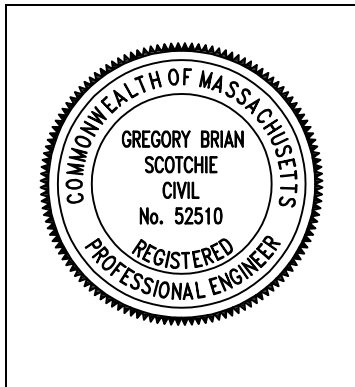
Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.


A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature




Signature and Date

5/3/2021

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

- New development
- Redevelopment
- Mix of New Development and Redevelopment

Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- No disturbance to any Wetland Resource Areas
- Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- Reduced Impervious Area (Redevelopment Only)
- Minimizing disturbance to existing trees and shrubs
- LID Site Design Credit Requested:
 - Credit 1
 - Credit 2
 - Credit 3
- Use of “country drainage” versus curb and gutter conveyance and pipe
- Bioretention Cells (includes Rain Gardens)
- Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- Treebox Filter
- Water Quality Swale
- Grass Channel
- Green Roof
- Other (describe): Infiltration Basins

Standard 1: No New Untreated Discharges

- No new untreated discharges
- Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.

Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

- Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

- Soil Analysis provided.
- Required Recharge Volume calculation provided.
- Required Recharge volume reduced through use of the LID site Design Credits.
- Sizing the infiltration, BMPs is based on the following method: Check the method used.
 - Static
 - Simple Dynamic
 - Dynamic Field¹
- Runoff from all impervious areas at the site discharging to the infiltration BMP.
- Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
 - Site is comprised solely of C and D soils and/or bedrock at the land surface
 - M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
 - Solid Waste Landfill pursuant to 310 CMR 19.000
 - Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
- Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.

Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

- The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
 - Provisions for storing materials and waste products inside or under cover;
 - Vehicle washing controls;
 - Requirements for routine inspections and maintenance of stormwater BMPs;
 - Spill prevention and response plans;
 - Provisions for maintenance of lawns, gardens, and other landscaped areas;
 - Requirements for storage and use of fertilizers, herbicides, and pesticides;
 - Pet waste management provisions;
 - Provisions for operation and management of septic systems;
 - Provisions for solid waste management;
 - Snow disposal and plowing plans relative to Wetland Resource Areas;
 - Winter Road Salt and/or Sand Use and Storage restrictions;
 - Street sweeping schedules;
 - Provisions for prevention of illicit discharges to the stormwater management system;
 - Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
 - Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
 - List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
 - Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
 - is within the Zone II or Interim Wellhead Protection Area
 - is near or to other critical areas
 - is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
 - involves runoff from land uses with higher potential pollutant loads.
 - The Required Water Quality Volume is reduced through use of the LID site Design Credits.
 - Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.

Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

- The BMP is sized (and calculations provided) based on:
 - The ½" or 1" Water Quality Volume or
 - The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted **prior to** the discharge of stormwater to the post-construction stormwater BMPs.
- The NPDES Multi-Sector General Permit does **not** cover the land use.
- LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- All exposure has been eliminated.
- All exposure has **not** been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

- The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- Critical areas and BMPs are identified in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

- The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:
 - Limited Project
 - Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
 - Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
 - Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
 - Bike Path and/or Foot Path
 - Redevelopment Project
 - Redevelopment portion of mix of new and redevelopment.
- Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
- The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
 - Construction Period Operation and Maintenance Plan;
 - Names of Persons or Entity Responsible for Plan Compliance;
 - Construction Period Pollution Prevention Measures;
 - Erosion and Sedimentation Control Plan Drawings;
 - Detail drawings and specifications for erosion control BMPs, including sizing calculations;
 - Vegetation Planning;
 - Site Development Plan;
 - Construction Sequencing Plan;
 - Sequencing of Erosion and Sedimentation Controls;
 - Operation and Maintenance of Erosion and Sedimentation Controls;
 - Inspection Schedule;
 - Maintenance Schedule;
 - Inspection and Maintenance Log Form.
- A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has **not** been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- The project is **not** covered by a NPDES Construction General Permit.
- The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

- The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
 - Name of the stormwater management system owners;
 - Party responsible for operation and maintenance;
 - Schedule for implementation of routine and non-routine maintenance tasks;
 - Plan showing the location of all stormwater BMPs maintenance access areas;
 - Description and delineation of public safety features;
 - Estimated operation and maintenance budget; and
 - Operation and Maintenance Log Form.
- The responsible party is **not** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
 - A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
 - A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

- The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- An Illicit Discharge Compliance Statement is attached;
- NO Illicit Discharge Compliance Statement is attached but will be submitted **prior to** the discharge of any stormwater to post-construction BMPs.

Stormwater Management System
Operation and Maintenance Plan
Long Term Pollution Prevention Plan

85 & 95 West Street
Northborough, MA

May 2021

PREPARED FOR:

Brant L. Viner & Margaret Harling
P.O. Box 295
Ellsworth, ME 04605

RESPONSIBILITY:

The applicant, or assigns will be responsible for implementation of the Operation and Maintenance Plan and Long Term Pollution Prevention Plan for the stormwater management system of a common driveway and associated infrastructure for five single family lots at 85 & 95 West Street and for any corrective action required.

A detailed, written log of all scheduled preventative and corrective maintenance performed for the stormwater management measures must be kept on site, including a record of all inspections and copies of maintenance-related work orders.

Attachment 1, "Inspection and Maintenance Check List" shall be maintained as a record of regularly scheduled inspection and maintenance items as outlined below for every year. Maintenance required and actions taken shall be recorded in Attachment 2, "Inspection and Maintenance Log". The funding, operation, and maintenance of all stormwater management Best Management Practices (BMPs) shall be provided by the Owner, or their appointee.

Maintenance routine and schedule: Routine inspections will be conducted on a monthly basis and thorough investigations will be conducted twice a year. Tasks that are common to all systems include regular removal of accumulated sediments, floatables and debris. Inspections will occur after every major storm event for the first six (6) months after construction. Inspections will be conducted by a qualified person experienced in drainage design and stormwater management systems. Annual notification to the Town Engineer before any maintenance work is performed is required and the Town Engineer shall be afforded the opportunity to inspect the work. Annual reports will be prepared detailing the status of the stormwater system and the maintenance performed. The Town Engineer shall be provided copies of the contract and invoices for all work performed. All material removed from the drainage system shall be properly disposed of off-site. A copy of the annual report will be sent to the Town of Northborough, if requested.

SITE CONDITIONS:

The stormwater management system for the site includes deep-sump, hooded catch basins; closed drainage system (pipe and manholes); stormwater treatment units (proprietary structures); surface detention basins; and subsurface infiltration systems.

DEEP SUMP CATCH BASINS:

1. Catch basins shall be inspected four (4) times per year and cleaned whenever depth of sediment is greater than twenty-four (24) inches.
2. All sediments and hydrocarbons shall be properly handled and disposed in accordance with local, state, and federal guidelines and regulations.

ALL SUBSURFACE INFILTRATION SYSTEMS: INF-103, INF-104, INF-105, INF-106, INF-201 and INF-RF2 THROUGH INF-RF6

1. At a minimum annual inspection are recommended. Initially, the system should be inspected every 6 months for the first year of operation. For subsequent years, the inspection should be adjusted based upon previous observation of sediment deposition.
2. Inspection ports are provided to allow for observation and cleaning of subsurface facilities. These covers may be buried, requiring clearing to access the infiltration system. The inspection ports allow for easy access to the system from the surface, eliminating the need to perform a confined space entry for inspection purposes.
3. Ponded water inside the infiltration facilities after several days may indicate the bottom of the system is clogged. If upon visual inspection it is found that sediment has accumulated, a stadia rod should be inserted to determine the depth of sediment. When the average depth of sediment exceeds 3 inches throughout the length of row, clean-out should be performed.
4. Downhill slopes from infiltration areas shall be monitored weekly during construction and monthly thereafter for the first year of operation for signs of breakout. Annual inspection is to occur after the first year. Seepage from downhill slopes shall be an indication of a system failure and corrective action shall be taken.
5. Corrective action for system failure shall include removal and replacement of all stone aggregate and filter fabric. Accumulated sediment shall be removed from the trench bottom and the bottom shall be scarified or tilled to help induce infiltration.
6. Maintenance is accomplished with the JetVac process. The JetVac process utilizes a high pressure water nozzle to propel itself down the row while scouring and suspending sediments. As the nozzle is retrieved, the captured pollutants are flushed back into the manhole for vacuuming. Most sewer and pipe maintenance companies have vacuum/JetVac combination vehicles. Selection of an appropriate JetVac nozzle will improve maintenance efficiency. Fixed nozzles designed for culverts or large diameter pipe cleaning are preferable. Rear facing jets with an effective spread of at least 45" are best. Most JetVac reels have 400 feet of hose allowing maintenance of an Isolator Row up to 50 chambers long.

7. Inspect isolator row for sediment
 - A. Inspection ports
 1. Remove/open lid on inline drain
 2. Remove and clean filter if installed
 3. Using a flashlight and stadia rod, measure depth of sediment and record on maintenance log
 4. Lower a camera into isolator row plus for visual inspection of sediment levels (optional)
 5. If sediment is at, or above, 3" (80 mm) proceed to step 2. If not, proceed to step 8.
 - B. All isolator rows
 1. Remove cover from structure at upstream end of isolator row
 2. Using a flashlight, inspect down the isolator row plus through outlet pipe mirrors on poles or cameras may be used to avoid a confined space entry follow osha regulations for confined space entry if entering manhole
 3. If sediment is at, or above, 3" (80 mm) proceed to step 2. If not, proceed to step 3.
8. Clean out isolator row using the jetvac process
 - C. A fixed culvert cleaning nozzle with rear facing spread of 45" (1.1 m) or more is preferred
 - D. Apply multiple passes of jetvac until backflush water is clean
 - E. Vacuum structure sump as required
9. Replace all covers, grates, filters, and lids; record observations and actions.
10. Inspect and clean basins and manholes upstream of the system.

EXTENDED DRY DETENTION BASIN:

1. The detention basin is to utilized as a temporary sediment trap during construction. Prior to permanent stabilization the basin is to be cleared to six (6") inches below finished grade and topped with clean loam. The basin(s) shall be inspected for accumulated sediment at least twice per year and sediment shall be removed when depth is 12 inches or at least once every 10 years.
2. Basins shall be inspected at least twice per year and immediately following large storm events to determine if the basin is functioning as intended. Inspections should be conducted during wet weather to determine if the basin is meeting the targeted detention times (24-hour average detention time; 72-hour drawdown). The basins shall be checked for slope integrity, soil moisture, vegetative health, soil stability, soil compaction, soil erosion, ponding, and sedimentation. Any necessary repairs shall be made immediately.
3. During the first few months following construction, the basin shall be inspected to ensure that the proposed vegetation becomes adequately established.
4. At least twice during the growing season, the basin, side slopes, and embankments shall be mowed and any accumulated trash and debris removed.
5. To maintain the dense growth of herbaceous vegetation, periodic reseeding may be required.
6. Basins shall not be used for snow removal and yard waste disposal.

7. Outlet control structures, headwalls, and riprap aprons or riprap stilling basins shall be checked a minimum of once per year for evidence of clogging or flow restrictions and cleared as necessary. Any debris or accumulated sediments which could hinder flows shall be removed and disposed.

SPILL CONTAINMENT:

1. In the event of a reportable spill, the Owner or its representative shall also be responsible for closing the cut-off valve in a timely manner and notifying the appropriate authorities of the spill. In the event that spill materials enter the stormwater management basin, the Owner shall be responsible for spill removal and restoration of the basin to its original condition in accordance with all applicable local and state regulations.
2. Pollutant transport and deposition may vary from year to year and regular inspections will help ensure that the system is cleaned out at the appropriate time. At a minimum, inspections should be performed twice per year (e.g. spring and fall) however more frequent inspections may be necessary in climates where winter sanding operations may lead to rapid accumulations, or in equipment washdown areas. Installations should also be inspected more frequently where excessive amounts of trash are expected.

LAWN/LANDSCAPE MAINTENANCE:

1. Apply fertilizers properly; at the proper time of year and at proper application rates to ensure absorption. Pesticides are to be applied by licensed applicators only.
2. Limit lawn watering: chose drought-tolerant landscaping and grasses, and use mulch and compost to retain moisture.
3. Under no circumstance shall the stormwater management system be used for yard waste and landscape debris.

DEICING:

1. The use and loading rates for application of deicing salts should be limited to the minimum required to maintain safe vehicular and pedestrian travel.
2. Alternative materials such as sand or gravel, calcium chloride, and calcium magnesium acetate should be considered in areas adjacent to stormwater management facilities and resource areas.
3. Deicing materials shall be covered to prevent loss and migration.
4. Deicing storage areas shall be located outside the 100-foot buffer zone to any wetlands or waterbodies. Ideally storage shall occur indoors.

SNOW MANAGEMENT:

1. Snow shall be stockpiled in pervious areas where it can slowly infiltrate. Under no circumstance shall the stormwater management system be used for snow storage.
2. Avoid dumping/piling snow over catch basins to prevent blockages and localized flooding of the drainage system.
3. The Owner shall be responsible to manage snow storage on-site and to ensure that snow is not stockpiled in the basin.
4. Sediments deposited from the snow storage areas shall be removed every spring.

SWEEPING OF PAVED SURFACES:

1. All paved surfaces on-site including driveways, loading areas, and parking areas shall be swept at least once annually to remove accumulations of sand, silt, leaves, and other debris.
2. Sweeping should occur in April/May after snowmelt has occurred and thaw has begun. Sweepings shall be disposed of an approved off-site location away from resource areas (wetlands or waterways) and stormwater management facilities.

Attachment 1

INSPECTION CHECKLIST 85 & 95 West Street, Northborough, MA 01532													
FOR YEAR:													
Inspection Item		Inspection Frequency*											
		Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sept	Oct	Nov	Dec
1	Subsurface Infiltration System												
	INF-103												
	INF-104												
	INF-105												
	INF-106												
	INF-RF2												
	INF-RF3												
	INF-RF4												
	INF-RF5												
	INF-RF6												
2	Drainage Pipes and Manholes	at least every five years											
3	Deep Sump Catch Basins & Manholes	at least four times per year											

* Actual time of inspecting and maintaining items may vary. Chart shall be used to indicate frequency of events

** This chart shall be used in conjunction with the attached "Stormwater Management Operation and Maintenance Plan" and to any conditions/special conditions as outlined in any Order of Conditions issued by the Northborough Conservation Commission and/or "Declaration of Easements, Covenants, Conditions and Restrictions" document.

Attachment 1

MAINTENANCE CHECKLIST - 85 & 95 West Street, Northborough, MA 01532													
FOR YEAR:													
Maintenance Item		Maintenance Item											
		Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sept	Oct	Nov	Dec
1	INF-103												
	INF-104												
	INF-105												
	INF-106												
	INF-RF2												
	INF-RF3												
	INF-RF4												
	INF-RF5												
	INF-RF6												
2	Drainage Pipes and Manholes	at least every five years											
3	Deep Sump Catch Basins & Manholes	at least four times per year											

* Actual time of maintaining items may vary. Chart shall be used to indicate frequency of events

** This chart shall be used in conjunction with the attached "Stormwater Management Operation and Maintenance Plan" and to any conditions/special conditions as outlined in any Order of Conditions issued by the Northborough Conservation Commission and/or "Declaration of Easements, Covenants, Conditions and Restrictions" document.

Name of Applicant: Brant L. Viner & Margaret Harling
Location: 85 & 95 West Street, Northborough, MA 01532

Inspection and Maintenance Log

Inspection No.	Date	Inspections Performed	Maintenance Action Taken
1			
2			
3			
4			
5			
6			
7			
8			
9			
10			
11			
12			
13			
14			
15			
16			
17			

Name of Applicant: Brant L. Viner & Margaret Harling
Location: 85 & 95 West Street, Northborough, MA 01532

Inspection and Maintenance Log

Inspection No.	Date	Inspections Performed	Maintenance Action Taken
18			
19			
20			
21			
22			
23			
24			
25			
26			
27			
28			
29			
30			
31			
32			

Calc. By: GBS
 Chk. By: _____

Date: 3-May-21
 Date: _____

**COMMON DRIVEWAY
 85 & 95 WEST STREET
 Northborough, MA**

Groundwater Recharge Calculations

Required Recharge Volume¹

Rv = F x impervious area

Where: Rv = required recharge volume (acre-feet)

NRCS Hydrologic Soil Type	Approx. Soil Texture	Target Depth Factor (inches)	Impervious Area (acre)	Rv (acre-feet)	Rv (cf)
A	sand	1.00	0.16	0.014	598
B	loam	0.35	0.00	0.000	0
C	silty loam	0.25	1.16	0.024	1,049
D	clay	0.10	0.00	0.000	1
Total =				0.038	1,648

Provided Recharge Volume²

Infiltration Area	Static Storage Volume (acre-feet)	Static Storage Volume (cf)
INF-103	0.0026	115
INF-104	0.0445	1,937
INF-105	0.1057	4,605
INF-106	0.0529	2,306
INF-201	0.0603	2,626
INF-RF2	0.0021	92
INF-RF3	0.0021	92
INF-RF4	0.0021	92
INF-RF5	0.0021	92
INF-RF6	0.0021	92
Pond-(1P)	0.0000	0
Total =	0.2766	12,049

Notes:

- 1.) Refer to Massachusetts Stormwater Handbook Volume 3, Chapter 1, page 15 dated February 2008.
- 2.) Provided recharge volume is based on the Static Method, refer to Massachusetts Stormwater Handbook Volume 3, Chapter 1, page 18 dated February 2008.

Calc. By: GBS

Date: 3-May-21

Chk. By: _____

Date: _____

**COMMON DRIVEWAY
85 & 95 WEST STREET
Northborough, MA**

Water Quality Calculations

WATER QUALITY TREATMENT VOLUME

$$V_{WQ} = (D_{WQ}/12 \text{ inches/foot}) * (A_{IMP} * 43,560 \text{ square feet/acre})$$

Where: V_{WQ} = Required Water Quality Volume (in cubic feet)

D_{WQ} = Water Quality Depth

A_{IMP} = Impervious Area (in acres)

Stormwater Facility	D_{WQ} (inches)	A_{IMP} (acres)	Required V_{WQ} (ft³)	Provided V_{WQ} (ft³)
INF-103	0.5	0.0944	171	115
INF-104	0.5	0.0644	117	1,937
INF-105	0.5	0.1933	351	4,605
INF-106	0.5	0.3012	547	2,306
INF-201	0.5	0.0636	115	2,626
INF-RF2	0.5	0.0715	130	92
INF-RF3	0.5	0.0387	70	92
INF-RF4	0.5	0.0715	130	92
INF-RF5	0.5	0.0715	130	92
INF-RF6	0.5	0.0715	130	92
Pond-(1P)	0.5	0.2391	434	0
Total =			2,325	12,049

Notes:

- 1.) Refer to Massachusetts Stormwater Handbook Volume 3, Chapter 1, page 32 dated February 2008.
- 2.) Refer to HydroCAD[®] report.

Rawls Rates

5/3/2021

Texture Class	NRCS Hydrologic Soil Group (HSG)	Infiltration Rate (inches/hour)
sand	A	8.27
loamy sand	A	2.41
sandy loam	B	1.02
loam	B	0.52
silt loam	C	0.27
sandy clay loam	C	0.17
clay loam	D	0.09
silty clay loam	D	0.06
sandy clay	D	0.05
silty clay	D	0.04
clay	D	0.02

Refer to Massachusetts Stormwater Handbook Volume 3, Chapter 1, page 22 dated February 2008.

Stage-Area-Storage for Pond INF-103: INF-103

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	
326.00	473	0	326.52	473	102	
326.01	473	2	326.53	473	105	
326.02	473	4	326.54	473	108	
326.03	473	6	326.55	473	112	Recharge
326.04	473	8	326.56	473	115	Volume
326.05	473	9	326.57	473	119	Provided
326.06	473	11	326.58	473	122	
326.07	473	13	326.59	473	126	
326.08	473	15	326.60	473	129	
326.09	473	17	326.61	473	133	
326.10	473	19	326.62	473	136	
326.11	473	21	326.63	473	140	
326.12	473	23	326.64	473	143	
326.13	473	25	326.65	473	147	
326.14	473	26	326.66	473	150	
326.15	473	28	326.67	473	153	
326.16	473	30	326.68	473	157	
326.17	473	32	326.69	473	160	
326.18	473	34	326.70	473	164	
326.19	473	36	326.71	473	167	
326.20	473	38	326.72	473	170	
326.21	473	40	326.73	473	174	
326.22	473	42	326.74	473	177	
326.23	473	43	326.75	473	180	
326.24	473	45	326.76	473	184	
326.25	473	47	326.77	473	187	
326.26	473	49	326.78	473	190	
326.27	473	51	326.79	473	194	
326.28	473	53	326.80	473	197	
326.29	473	55	326.81	473	200	
326.30	473	57	326.82	473	203	
326.31	473	59	326.83	473	207	
326.32	473	61	326.84	473	210	
326.33	473	62	326.85	473	213	
326.34	473	64	326.86	473	216	
326.35	473	66	326.87	473	220	
326.36	473	68	326.88	473	223	
326.37	473	70	326.89	473	226	
326.38	473	72	326.90	473	229	
326.39	473	74	326.91	473	232	
326.40	473	76	326.92	473	236	
326.41	473	78	326.93	473	239	
326.42	473	79	326.94	473	242	
326.43	473	81	326.95	473	245	
326.44	473	83	326.96	473	248	
326.45	473	85	326.97	473	251	
326.46	473	87	326.98	473	254	
326.47	473	89	326.99	473	258	
326.48	473	91	327.00	473	261	
326.49	473	93	327.01	473	264	
326.50	473	95	327.02	473	267	
326.51	473	98	327.03	473	270	

Stage-Area-Storage for Pond INF-104: INF-104 (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
326.14	1,889	1,205	326.66	1,889	1,966
326.15	1,889	1,220	326.67	1,889	1,980
326.16	1,889	1,235	326.68	1,889	1,994
326.17	1,889	1,250	326.69	1,889	2,008
326.18	1,889	1,265	326.70	1,889	2,022
326.19	1,889	1,280	326.71	1,889	2,037
326.20	1,889	1,295	326.72	1,889	2,051
326.21	1,889	1,310	326.73	1,889	2,065
326.22	1,889	1,325	326.74	1,889	2,079
326.23	1,889	1,340	326.75	1,889	2,093
326.24	1,889	1,355	326.76	1,889	2,107
326.25	1,889	1,369	326.77	1,889	2,121
326.26	1,889	1,384	326.78	1,889	2,135
326.27	1,889	1,399	326.79	1,889	2,148
326.28	1,889	1,414	326.80	1,889	2,162
326.29	1,889	1,429	326.81	1,889	2,176
326.30	1,889	1,444	326.82	1,889	2,190
326.31	1,889	1,458	326.83	1,889	2,204
326.32	1,889	1,473	326.84	1,889	2,218
326.33	1,889	1,488	326.85	1,889	2,232
326.34	1,889	1,503	326.86	1,889	2,245
326.35	1,889	1,517	326.87	1,889	2,259
326.36	1,889	1,532	326.88	1,889	2,273
326.37	1,889	1,547	326.89	1,889	2,286
326.38	1,889	1,561	326.90	1,889	2,300
326.39	1,889	1,576	326.91	1,889	2,314
326.40	1,889	1,591	326.92	1,889	2,327
326.41	1,889	1,605	326.93	1,889	2,341
326.42	1,889	1,620	326.94	1,889	2,355
326.43	1,889	1,635	326.95	1,889	2,368
326.44	1,889	1,649	326.96	1,889	2,382
326.45	1,889	1,664	326.97	1,889	2,395
326.46	1,889	1,678	326.98	1,889	2,409
326.47	1,889	1,693	326.99	1,889	2,422
326.48	1,889	1,707	327.00	1,889	2,435
326.49	1,889	1,722	327.01	1,889	2,449
326.50	1,889	1,736	327.02	1,889	2,462
326.51	1,889	1,751	327.03	1,889	2,476
326.52	1,889	1,765	327.04	1,889	2,489
326.53	1,889	1,780	327.05	1,889	2,502
326.54	1,889	1,794	327.06	1,889	2,515
326.55	1,889	1,809	327.07	1,889	2,529
326.56	1,889	1,823	327.08	1,889	2,542
326.57	1,889	1,837	327.09	1,889	2,555
326.58	1,889	1,852	327.10	1,889	2,568
326.59	1,889	1,866	327.11	1,889	2,581
326.60	1,889	1,880	327.12	1,889	2,594
326.61	1,889	1,895	327.13	1,889	2,607
326.62	1,889	1,909	327.14	1,889	2,621
326.63	1,889	1,923	327.15	1,889	2,634
Recharge Volume Provided 326.64	1,889	1,937	327.16	1,889	2,647
326.65	1,889	1,952	327.17	1,889	2,660

Stage-Area-Storage for Pond INF-105: INF-105 (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	
347.54	3,941	2,545	348.06	3,941	4,159	
347.55	3,941	2,577	348.07	3,941	4,189	
347.56	3,941	2,608	348.08	3,941	4,219	
347.57	3,941	2,640	348.09	3,941	4,249	
347.58	3,941	2,672	348.10	3,941	4,279	
347.59	3,941	2,704	348.11	3,941	4,309	
347.60	3,941	2,735	348.12	3,941	4,339	
347.61	3,941	2,767	348.13	3,941	4,369	
347.62	3,941	2,799	348.14	3,941	4,398	
347.63	3,941	2,830	348.15	3,941	4,428	
347.64	3,941	2,862	348.16	3,941	4,458	
347.65	3,941	2,894	348.17	3,941	4,487	
347.66	3,941	2,925	348.18	3,941	4,517	
347.67	3,941	2,957	348.19	3,941	4,546	
347.68	3,941	2,988	348.20	3,941	4,576	
347.69	3,941	3,020	348.21	3,941	4,605	Recharge
347.70	3,941	3,051	348.22	3,941	4,634	Volume
347.71	3,941	3,082	348.23	3,941	4,664	Provided
347.72	3,941	3,114	348.24	3,941	4,693	
347.73	3,941	3,145	348.25	3,941	4,722	
347.74	3,941	3,176	348.26	3,941	4,751	
347.75	3,941	3,208	348.27	3,941	4,781	
347.76	3,941	3,239	348.28	3,941	4,810	
347.77	3,941	3,270	348.29	3,941	4,839	
347.78	3,941	3,301	348.30	3,941	4,868	
347.79	3,941	3,332	348.31	3,941	4,896	
347.80	3,941	3,363	348.32	3,941	4,925	
347.81	3,941	3,395	348.33	3,941	4,954	
347.82	3,941	3,426	348.34	3,941	4,983	
347.83	3,941	3,457	348.35	3,941	5,012	
347.84	3,941	3,487	348.36	3,941	5,040	
347.85	3,941	3,518	348.37	3,941	5,069	
347.86	3,941	3,549	348.38	3,941	5,097	
347.87	3,941	3,580	348.39	3,941	5,126	
347.88	3,941	3,611	348.40	3,941	5,154	
347.89	3,941	3,642	348.41	3,941	5,183	
347.90	3,941	3,672	348.42	3,941	5,211	
347.91	3,941	3,703	348.43	3,941	5,239	
347.92	3,941	3,734	348.44	3,941	5,267	
347.93	3,941	3,764	348.45	3,941	5,296	
347.94	3,941	3,795	348.46	3,941	5,324	
347.95	3,941	3,826	348.47	3,941	5,352	
347.96	3,941	3,856	348.48	3,941	5,380	
347.97	3,941	3,887	348.49	3,941	5,408	
347.98	3,941	3,917	348.50	3,941	5,435	
347.99	3,941	3,947	348.51	3,941	5,463	
348.00	3,941	3,978	348.52	3,941	5,491	
348.01	3,941	4,008	348.53	3,941	5,519	
348.02	3,941	4,038	348.54	3,941	5,546	
348.03	3,941	4,069	348.55	3,941	5,574	
348.04	3,941	4,099	348.56	3,941	5,601	
348.05	3,941	4,129	348.57	3,941	5,629	

Stage-Area-Storage for Pond INF-106: INF-106 (continued)

	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
	355.41	786	2,235	355.93	786	2,491
	355.42	786	2,240	355.94	786	2,496
	355.43	786	2,245	355.95	786	2,500
	355.44	786	2,250	355.96	786	2,505
	355.45	786	2,255	355.97	786	2,510
	355.46	786	2,260	355.98	786	2,514
	355.47	786	2,265	355.99	786	2,519
	355.48	786	2,270	356.00	786	2,524
	355.49	786	2,275	356.01	786	2,528
	355.50	786	2,281	356.02	786	2,533
	355.51	786	2,286	356.03	786	2,537
	355.52	786	2,291	356.04	786	2,542
	355.53	786	2,296	356.05	786	2,547
Recharge	355.54	786	2,301	356.06	786	2,551
Volume	355.55	786	2,306	356.07	786	2,556
Provided	355.56	786	2,311	356.08	786	2,560
	355.57	786	2,316	356.09	786	2,565
	355.58	786	2,321	356.10	786	2,569
	355.59	786	2,326	356.11	786	2,574
	355.60	786	2,331	356.12	786	2,578
	355.61	786	2,336	356.13	786	2,583
	355.62	786	2,341	356.14	786	2,587
	355.63	786	2,346	356.15	786	2,592
	355.64	786	2,351	356.16	786	2,596
	355.65	786	2,356	356.17	786	2,601
	355.66	786	2,361	356.18	786	2,605
	355.67	786	2,366	356.19	786	2,610
	355.68	786	2,370	356.20	786	2,614
	355.69	786	2,375	356.21	786	2,618
	355.70	786	2,380	356.22	786	2,623
	355.71	786	2,385	356.23	786	2,627
	355.72	786	2,390	356.24	786	2,631
	355.73	786	2,395	356.25	786	2,636
	355.74	786	2,400	356.26	786	2,640
	355.75	786	2,405	356.27	786	2,644
	355.76	786	2,410	356.28	786	2,649
	355.77	786	2,414	356.29	786	2,653
	355.78	786	2,419	356.30	786	2,657
	355.79	786	2,424	356.31	786	2,662
	355.80	786	2,429	356.32	786	2,666
	355.81	786	2,434	356.33	786	2,670
	355.82	786	2,439	356.34	786	2,674
	355.83	786	2,443	356.35	786	2,678
	355.84	786	2,448	356.36	786	2,683
	355.85	786	2,453	356.37	786	2,687
	355.86	786	2,458	356.38	786	2,691
	355.87	786	2,463	356.39	786	2,695
	355.88	786	2,467	356.40	786	2,699
	355.89	786	2,472	356.41	786	2,703
	355.90	786	2,477	356.42	786	2,707
	355.91	786	2,482	356.43	786	2,711
	355.92	786	2,486	356.44	786	2,715

Stage-Area-Storage for Pond INF-201: INF-201 (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
337.24	3,236	2,084	337.76	3,236	3,405
337.25	3,236	2,110	337.77	3,236	3,429
337.26	3,236	2,136	337.78	3,236	3,454
337.27	3,236	2,162	337.79	3,236	3,479
337.28	3,236	2,188	337.80	3,236	3,503
337.29	3,236	2,214	337.81	3,236	3,527
337.30	3,236	2,240	337.82	3,236	3,552
337.31	3,236	2,266	337.83	3,236	3,576
337.32	3,236	2,292	337.84	3,236	3,601
337.33	3,236	2,318	337.85	3,236	3,625
337.34	3,236	2,344	337.86	3,236	3,649
337.35	3,236	2,370	337.87	3,236	3,673
337.36	3,236	2,395	337.88	3,236	3,697
337.37	3,236	2,421	337.89	3,236	3,722
337.38	3,236	2,447	337.90	3,236	3,746
337.39	3,236	2,473	337.91	3,236	3,770
337.40	3,236	2,498	337.92	3,236	3,794
337.41	3,236	2,524	337.93	3,236	3,818
337.42	3,236	2,550	337.94	3,236	3,842
337.43	3,236	2,575	337.95	3,236	3,866
337.44	3,236	2,601	337.96	3,236	3,889
Recharge Volume Provided 337.45	3,236	2,626	337.97	3,236	3,913
337.46	3,236	2,652	337.98	3,236	3,937
337.47	3,236	2,677	337.99	3,236	3,961
337.48	3,236	2,703	338.00	3,236	3,985
337.49	3,236	2,728	338.01	3,236	4,008
337.50	3,236	2,754	338.02	3,236	4,032
337.51	3,236	2,779	338.03	3,236	4,055
337.52	3,236	2,805	338.04	3,236	4,079
337.53	3,236	2,830	338.05	3,236	4,102
337.54	3,236	2,855	338.06	3,236	4,126
337.55	3,236	2,881	338.07	3,236	4,149
337.56	3,236	2,906	338.08	3,236	4,173
337.57	3,236	2,931	338.09	3,236	4,196
337.58	3,236	2,956	338.10	3,236	4,219
337.59	3,236	2,981	338.11	3,236	4,242
337.60	3,236	3,007	338.12	3,236	4,266
337.61	3,236	3,032	338.13	3,236	4,289
337.62	3,236	3,057	338.14	3,236	4,312
337.63	3,236	3,082	338.15	3,236	4,335
337.64	3,236	3,107	338.16	3,236	4,358
337.65	3,236	3,132	338.17	3,236	4,381
337.66	3,236	3,157	338.18	3,236	4,404
337.67	3,236	3,182	338.19	3,236	4,427
337.68	3,236	3,207	338.20	3,236	4,449
337.69	3,236	3,232	338.21	3,236	4,472
337.70	3,236	3,256	338.22	3,236	4,495
337.71	3,236	3,281	338.23	3,236	4,517
337.72	3,236	3,306	338.24	3,236	4,540
337.73	3,236	3,331	338.25	3,236	4,563
337.74	3,236	3,356	338.26	3,236	4,585
337.75	3,236	3,380	338.27	3,236	4,608

Stage-Area-Storage for Pond INF-RF2: INF-RF2 (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
335.04	118	66	335.56	118	105
335.05	118	67	335.57	118	106
335.06	118	67	335.58	118	107
335.07	118	68	335.59	118	107
335.08	118	69	335.60	118	108
335.09	118	70	335.61	118	109
335.10	118	71	335.62	118	110
335.11	118	71	335.63	118	110
335.12	118	72	335.64	118	111
335.13	118	73	335.65	118	112
335.14	118	74	335.66	118	113
335.15	118	74	335.67	118	113
335.16	118	75	335.68	118	114
335.17	118	76	335.69	118	115
335.18	118	77	335.70	118	115
335.19	118	77	335.71	118	116
335.20	118	78	335.72	118	117
335.21	118	79	335.73	118	118
335.22	118	80	335.74	118	118
335.23	118	80	335.75	118	119
335.24	118	81	335.76	118	120
335.25	118	82	335.77	118	120
335.26	118	83	335.78	118	121
335.27	118	83	335.79	118	122
335.28	118	84	335.80	118	123
335.29	118	85	335.81	118	123
335.30	118	86	335.82	118	124
335.31	118	87	335.83	118	125
335.32	118	87	335.84	118	125
335.33	118	88	335.85	118	126
335.34	118	89	335.86	118	127
335.35	118	90	335.87	118	128
335.36	118	90	335.88	118	128
335.37	118	91	335.89	118	129
Recharge Volume Provided	335.38	118	92	335.90	130
	335.39	118	93	335.91	130
	335.40	118	93	335.92	131
	335.41	118	94	335.93	132
	335.42	118	95	335.94	133
	335.43	118	96	335.95	133
	335.44	118	96	335.96	134
	335.45	118	97	335.97	135
	335.46	118	98	335.98	135
	335.47	118	99	335.99	136
	335.48	118	99	336.00	137
	335.49	118	100	336.01	137
	335.50	118	101	336.02	138
	335.51	118	101	336.03	139
	335.52	118	102	336.04	140
	335.53	118	103	336.05	140
	335.54	118	104	336.06	141
	335.55	118	104	336.07	142

Stage-Area-Storage for Pond INF-RF3: INF-RF3 (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
333.04	118	66	333.56	118	105
333.05	118	67	333.57	118	106
333.06	118	67	333.58	118	107
333.07	118	68	333.59	118	107
333.08	118	69	333.60	118	108
333.09	118	70	333.61	118	109
333.10	118	71	333.62	118	110
333.11	118	71	333.63	118	110
333.12	118	72	333.64	118	111
333.13	118	73	333.65	118	112
333.14	118	74	333.66	118	113
333.15	118	74	333.67	118	113
333.16	118	75	333.68	118	114
333.17	118	76	333.69	118	115
333.18	118	77	333.70	118	115
333.19	118	77	333.71	118	116
333.20	118	78	333.72	118	117
333.21	118	79	333.73	118	118
333.22	118	80	333.74	118	118
333.23	118	80	333.75	118	119
333.24	118	81	333.76	118	120
333.25	118	82	333.77	118	120
333.26	118	83	333.78	118	121
333.27	118	83	333.79	118	122
333.28	118	84	333.80	118	123
333.29	118	85	333.81	118	123
333.30	118	86	333.82	118	124
333.31	118	87	333.83	118	125
333.32	118	87	333.84	118	125
333.33	118	88	333.85	118	126
333.34	118	89	333.86	118	127
333.35	118	90	333.87	118	128
333.36	118	90	333.88	118	128
Recharge 333.37	118	91	333.89	118	129
Volume 333.38	118	92	333.90	118	130
Provided 333.39	118	93	333.91	118	130
333.40	118	93	333.92	118	131
333.41	118	94	333.93	118	132
333.42	118	95	333.94	118	133
333.43	118	96	333.95	118	133
333.44	118	96	333.96	118	134
333.45	118	97	333.97	118	135
333.46	118	98	333.98	118	135
333.47	118	99	333.99	118	136
333.48	118	99	334.00	118	137
333.49	118	100	334.01	118	137
333.50	118	101	334.02	118	138
333.51	118	101	334.03	118	139
333.52	118	102	334.04	118	140
333.53	118	103	334.05	118	140
333.54	118	104	334.06	118	141
333.55	118	104	334.07	118	142

Stage-Area-Storage for Pond INF-RF4: INF-RF4 (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
367.04	118	66	367.56	118	105
367.05	118	67	367.57	118	106
367.06	118	67	367.58	118	107
367.07	118	68	367.59	118	107
367.08	118	69	367.60	118	108
367.09	118	70	367.61	118	109
367.10	118	71	367.62	118	110
367.11	118	71	367.63	118	110
367.12	118	72	367.64	118	111
367.13	118	73	367.65	118	112
367.14	118	74	367.66	118	113
367.15	118	74	367.67	118	113
367.16	118	75	367.68	118	114
367.17	118	76	367.69	118	115
367.18	118	77	367.70	118	115
367.19	118	77	367.71	118	116
367.20	118	78	367.72	118	117
367.21	118	79	367.73	118	118
367.22	118	80	367.74	118	118
367.23	118	80	367.75	118	119
367.24	118	81	367.76	118	120
367.25	118	82	367.77	118	120
367.26	118	83	367.78	118	121
367.27	118	83	367.79	118	122
367.28	118	84	367.80	118	123
367.29	118	85	367.81	118	123
367.30	118	86	367.82	118	124
367.31	118	87	367.83	118	125
367.32	118	87	367.84	118	125
367.33	118	88	367.85	118	126
367.34	118	89	367.86	118	127
367.35	118	90	367.87	118	128
367.36	118	90	367.88	118	128
367.37	118	91	367.89	118	129
Recharge Volume Provided	367.38	118	92	367.90	130
	367.39	118	93	367.91	130
	367.40	118	93	367.92	131
	367.41	118	94	367.93	132
	367.42	118	95	367.94	133
	367.43	118	96	367.95	133
	367.44	118	96	367.96	134
	367.45	118	97	367.97	135
	367.46	118	98	367.98	135
	367.47	118	99	367.99	136
	367.48	118	99	368.00	137
	367.49	118	100	368.01	137
	367.50	118	101	368.02	138
	367.51	118	101	368.03	139
	367.52	118	102	368.04	140
	367.53	118	103	368.05	140
	367.54	118	104	368.06	141
	367.55	118	104	368.07	142

Stage-Area-Storage for Pond INF-RF5: INF-RF5 (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
377.04	118	66	377.56	118	105
377.05	118	67	377.57	118	106
377.06	118	67	377.58	118	107
377.07	118	68	377.59	118	107
377.08	118	69	377.60	118	108
377.09	118	70	377.61	118	109
377.10	118	71	377.62	118	110
377.11	118	71	377.63	118	110
377.12	118	72	377.64	118	111
377.13	118	73	377.65	118	112
377.14	118	74	377.66	118	113
377.15	118	74	377.67	118	113
377.16	118	75	377.68	118	114
377.17	118	76	377.69	118	115
377.18	118	77	377.70	118	115
377.19	118	77	377.71	118	116
377.20	118	78	377.72	118	117
377.21	118	79	377.73	118	118
377.22	118	80	377.74	118	118
377.23	118	80	377.75	118	119
377.24	118	81	377.76	118	120
377.25	118	82	377.77	118	120
377.26	118	83	377.78	118	121
377.27	118	83	377.79	118	122
377.28	118	84	377.80	118	123
377.29	118	85	377.81	118	123
377.30	118	86	377.82	118	124
377.31	118	87	377.83	118	125
377.32	118	87	377.84	118	125
377.33	118	88	377.85	118	126
377.34	118	89	377.86	118	127
377.35	118	90	377.87	118	128
377.36	118	90	377.88	118	128
377.37	118	91	377.89	118	129
377.38	118	92	377.90	118	130
377.39	118	93	377.91	118	130
377.40	118	93	377.92	118	131
377.41	118	94	377.93	118	132
377.42	118	95	377.94	118	133
377.43	118	96	377.95	118	133
377.44	118	96	377.96	118	134
377.45	118	97	377.97	118	135
377.46	118	98	377.98	118	135
377.47	118	99	377.99	118	136
377.48	118	99	378.00	118	137
377.49	118	100	378.01	118	137
377.50	118	101	378.02	118	138
377.51	118	101	378.03	118	139
377.52	118	102	378.04	118	140
377.53	118	103	378.05	118	140
377.54	118	104	378.06	118	141
377.55	118	104	378.07	118	142

Recharge
Volume
Provided

Stage-Area-Storage for Pond INF-RF6: INF-RF6 (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
377.04	118	66	377.56	118	105
377.05	118	67	377.57	118	106
377.06	118	67	377.58	118	107
377.07	118	68	377.59	118	107
377.08	118	69	377.60	118	108
377.09	118	70	377.61	118	109
377.10	118	71	377.62	118	110
377.11	118	71	377.63	118	110
377.12	118	72	377.64	118	111
377.13	118	73	377.65	118	112
377.14	118	74	377.66	118	113
377.15	118	74	377.67	118	113
377.16	118	75	377.68	118	114
377.17	118	76	377.69	118	115
377.18	118	77	377.70	118	115
377.19	118	77	377.71	118	116
377.20	118	78	377.72	118	117
377.21	118	79	377.73	118	118
377.22	118	80	377.74	118	118
377.23	118	80	377.75	118	119
377.24	118	81	377.76	118	120
377.25	118	82	377.77	118	120
377.26	118	83	377.78	118	121
377.27	118	83	377.79	118	122
377.28	118	84	377.80	118	123
377.29	118	85	377.81	118	123
377.30	118	86	377.82	118	124
377.31	118	87	377.83	118	125
377.32	118	87	377.84	118	125
377.33	118	88	377.85	118	126
377.34	118	89	377.86	118	127
377.35	118	90	377.87	118	128
377.36	118	90	377.88	118	128
377.37	118	91	377.89	118	129
377.38	118	92	377.90	118	130
377.39	118	93	377.91	118	130
377.40	118	93	377.92	118	131
377.41	118	94	377.93	118	132
377.42	118	95	377.94	118	133
377.43	118	96	377.95	118	133
377.44	118	96	377.96	118	134
377.45	118	97	377.97	118	135
377.46	118	98	377.98	118	135
377.47	118	99	377.99	118	136
377.48	118	99	378.00	118	137
377.49	118	100	378.01	118	137
377.50	118	101	378.02	118	138
377.51	118	101	378.03	118	139
377.52	118	102	378.04	118	140
377.53	118	103	378.05	118	140
377.54	118	104	378.06	118	141
377.55	118	104	378.07	118	142

Recharge
Volume
Provided

Hydrograph for Pond INF-103: INF-103

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Outflow (cfs)	Discarded (cfs)	Primary (cfs)
0.00	0.0	0	326.00	0.0	0.0	0.0
2.00	0.0	0	326.00	0.0	0.0	0.0
4.00	0.0	0	326.00	0.0	0.0	0.0
6.00	0.0	7	326.04	0.0	0.0	0.0
8.00	0.0	65	326.34	0.0	0.0	0.0
10.00	0.0	136	326.62	0.0	0.0	0.0
12.00	2.2	281	327.07	1.9	0.0	1.9
14.00	0.3	166	326.71	0.3	0.0	0.3
16.00	0.1	150	326.66	0.1	0.0	0.1
18.00	0.1	139	326.63	0.1	0.0	0.1
20.00	0.0	135	326.62	0.0	0.0	0.0
22.00	0.0	131	326.61	0.0	0.0	0.0
24.00	0.0	127	326.59	0.0	0.0	0.0
26.00	0.0	101	326.52	0.0	0.0	0.0
28.00	0.0	79	326.42	0.0	0.0	0.0
30.00	0.0	57	326.30	0.0	0.0	0.0
32.00	0.0	36	326.19	0.0	0.0	0.0
34.00	0.0	14	326.07	0.0	0.0	0.0
36.00	0.0	0	326.00	0.0	0.0	0.0
38.00	0.0	0	326.00	0.0	0.0	0.0
40.00	0.0	0	326.00	0.0	0.0	0.0
42.00	0.0	0	326.00	0.0	0.0	0.0
44.00	0.0	0	326.00	0.0	0.0	0.0
46.00	0.0	0	326.00	0.0	0.0	0.0
48.00	0.0	0	326.00	0.0	0.0	0.0
50.00	0.0	0	326.00	0.0	0.0	0.0
52.00	0.0	0	326.00	0.0	0.0	0.0
54.00	0.0	0	326.00	0.0	0.0	0.0
56.00	0.0	0	326.00	0.0	0.0	0.0
58.00	0.0	0	326.00	0.0	0.0	0.0
60.00	0.0	0	326.00	0.0	0.0	0.0
62.00	0.0	0	326.00	0.0	0.0	0.0
64.00	0.0	0	326.00	0.0	0.0	0.0
66.00	0.0	0	326.00	0.0	0.0	0.0
68.00	0.0	0	326.00	0.0	0.0	0.0
70.00	0.0	0	326.00	0.0	0.0	0.0
72.00	0.0	0	326.00	0.0	0.0	0.0
74.00	0.0	0	326.00	0.0	0.0	0.0
76.00	0.0	0	326.00	0.0	0.0	0.0
78.00	0.0	0	326.00	0.0	0.0	0.0
80.00	0.0	0	326.00	0.0	0.0	0.0
82.00	0.0	0	326.00	0.0	0.0	0.0
84.00	0.0	0	326.00	0.0	0.0	0.0
86.00	0.0	0	326.00	0.0	0.0	0.0
88.00	0.0	0	326.00	0.0	0.0	0.0
90.00	0.0	0	326.00	0.0	0.0	0.0
92.00	0.0	0	326.00	0.0	0.0	0.0
94.00	0.0	0	326.00	0.0	0.0	0.0
96.00	0.0	0	326.00	0.0	0.0	0.0

Drawdown Time < 72 Hours

Hydrograph for Pond INF-104: INF-104

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Outflow (cfs)	Discarded (cfs)	Primary (cfs)
0.00	0.0	0	325.10	0.0	0.0	0.0
2.00	0.0	0	325.10	0.0	0.0	0.0
4.00	0.0	0	325.10	0.0	0.0	0.0
6.00	0.0	0	325.10	0.0	0.0	0.0
8.00	0.0	2	325.10	0.0	0.0	0.0
10.00	0.2	23	325.13	0.1	0.1	0.0
12.00	2.8	2,651	327.16	1.6	0.1	1.5
14.00	1.3	2,581	327.11	1.3	0.1	1.2
16.00	0.6	2,390	326.97	0.7	0.1	0.5
18.00	0.3	2,251	326.86	0.4	0.1	0.2
20.00	0.2	2,156	326.80	0.2	0.1	0.1
22.00	0.2	2,083	326.74	0.2	0.1	0.0
24.00	0.1	1,981	326.67	0.1	0.1	0.0
26.00	0.0	1,137	326.10	0.1	0.1	0.0
28.00	0.0	170	325.33	0.1	0.1	0.0
30.00	0.0	0	325.10	0.0	0.0	0.0
32.00	0.0	0	325.10	0.0	0.0	0.0
34.00	0.0	0	325.10	0.0	0.0	0.0
36.00	0.0	0	325.10	0.0	0.0	0.0
38.00	0.0	0	325.10	0.0	0.0	0.0
40.00	0.0	0	325.10	0.0	0.0	0.0
42.00	0.0	0	325.10	0.0	0.0	0.0
44.00	0.0	0	325.10	0.0	0.0	0.0
46.00	0.0	0	325.10	0.0	0.0	0.0
48.00	0.0	0	325.10	0.0	0.0	0.0
50.00	0.0	0	325.10	0.0	0.0	0.0
52.00	0.0	0	325.10	0.0	0.0	0.0
54.00	0.0	0	325.10	0.0	0.0	0.0
56.00	0.0	0	325.10	0.0	0.0	0.0
58.00	0.0	0	325.10	0.0	0.0	0.0
60.00	0.0	0	325.10	0.0	0.0	0.0
62.00	0.0	0	325.10	0.0	0.0	0.0
64.00	0.0	0	325.10	0.0	0.0	0.0
66.00	0.0	0	325.10	0.0	0.0	0.0
68.00	0.0	0	325.10	0.0	0.0	0.0
70.00	0.0	0	325.10	0.0	0.0	0.0
72.00	0.0	0	325.10	0.0	0.0	0.0
74.00	0.0	0	325.10	0.0	0.0	0.0
76.00	0.0	0	325.10	0.0	0.0	0.0
78.00	0.0	0	325.10	0.0	0.0	0.0
80.00	0.0	0	325.10	0.0	0.0	0.0
82.00	0.0	0	325.10	0.0	0.0	0.0
84.00	0.0	0	325.10	0.0	0.0	0.0
86.00	0.0	0	325.10	0.0	0.0	0.0
88.00	0.0	0	325.10	0.0	0.0	0.0
90.00	0.0	0	325.10	0.0	0.0	0.0
92.00	0.0	0	325.10	0.0	0.0	0.0
94.00	0.0	0	325.10	0.0	0.0	0.0
96.00	0.0	0	325.10	0.0	0.0	0.0

Drawdown Time < 72 Hours

Hydrograph for Pond INF-105: INF-105

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Outflow (cfs)	Discarded (cfs)	Primary (cfs)
0.00	0.0	0	346.50	0.0	0.0	0.0
2.00	0.0	0	346.50	0.0	0.0	0.0
4.00	0.0	0	346.50	0.0	0.0	0.0
6.00	0.0	0	346.50	0.0	0.0	0.0
8.00	0.0	2	346.50	0.0	0.0	0.0
10.00	0.2	104	346.57	0.1	0.1	0.0
12.00	2.7	3,581	347.87	0.1	0.1	0.0
14.00	0.8	5,729	348.61	0.8	0.1	0.7
16.00	0.4	5,425	348.50	0.5	0.1	0.3
18.00	0.3	5,198	348.42	0.3	0.1	0.2
20.00	0.2	5,052	348.36	0.2	0.1	0.1
22.00	0.2	4,956	348.33	0.2	0.1	0.1
24.00	0.1	4,853	348.29	0.1	0.1	0.0
26.00	0.0	4,120	348.05	0.1	0.1	0.0
28.00	0.0	3,277	347.77	0.1	0.1	0.0
30.00	0.0	2,433	347.51	0.1	0.1	0.0
32.00	0.0	1,590	347.24	0.1	0.1	0.0
34.00	0.0	747	346.97	0.1	0.1	0.0
36.00	0.0	0	346.50	0.0	0.0	0.0
38.00	0.0	0	346.50	0.0	0.0	0.0
40.00	0.0	0	346.50	0.0	0.0	0.0
42.00	0.0	0	346.50	0.0	0.0	0.0
44.00	0.0	0	346.50	0.0	0.0	0.0
46.00	0.0	0	346.50	0.0	0.0	0.0
48.00	0.0	0	346.50	0.0	0.0	0.0
50.00	0.0	0	346.50	0.0	0.0	0.0
52.00	0.0	0	346.50	0.0	0.0	0.0
54.00	0.0	0	346.50	0.0	0.0	0.0
56.00	0.0	0	346.50	0.0	0.0	0.0
58.00	0.0	0	346.50	0.0	0.0	0.0
60.00	0.0	0	346.50	0.0	0.0	0.0
62.00	0.0	0	346.50	0.0	0.0	0.0
64.00	0.0	0	346.50	0.0	0.0	0.0
66.00	0.0	0	346.50	0.0	0.0	0.0
68.00	0.0	0	346.50	0.0	0.0	0.0
70.00	0.0	0	346.50	0.0	0.0	0.0
72.00	0.0	0	346.50	0.0	0.0	0.0
74.00	0.0	0	346.50	0.0	0.0	0.0
76.00	0.0	0	346.50	0.0	0.0	0.0
78.00	0.0	0	346.50	0.0	0.0	0.0
80.00	0.0	0	346.50	0.0	0.0	0.0
82.00	0.0	0	346.50	0.0	0.0	0.0
84.00	0.0	0	346.50	0.0	0.0	0.0
86.00	0.0	0	346.50	0.0	0.0	0.0
88.00	0.0	0	346.50	0.0	0.0	0.0
90.00	0.0	0	346.50	0.0	0.0	0.0
92.00	0.0	0	346.50	0.0	0.0	0.0
94.00	0.0	0	346.50	0.0	0.0	0.0
96.00	0.0	0	346.50	0.0	0.0	0.0

Drawdown Time < 72 Hours

Hydrograph for Pond INF-106: INF-106

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Outflow (cfs)	Discarded (cfs)	Primary (cfs)
0.00	0.0	0	351.25	0.0	0.0	0.0
2.00	0.0	0	351.25	0.0	0.0	0.0
4.00	0.0	0	351.25	0.0	0.0	0.0
6.00	0.0	2	351.26	0.0	0.0	0.0
8.00	0.1	48	351.40	0.0	0.0	0.0
10.00	0.1	490	352.41	0.0	0.0	0.0
12.00	2.1	2,673	356.34	1.4	0.0	1.4
14.00	0.2	2,394	355.73	0.2	0.0	0.2
16.00	0.1	2,364	355.67	0.1	0.0	0.1
18.00	0.1	2,345	355.63	0.1	0.0	0.0
20.00	0.1	2,336	355.61	0.1	0.0	0.0
22.00	0.0	2,329	355.60	0.0	0.0	0.0
24.00	0.0	2,319	355.58	0.0	0.0	0.0
26.00	0.0	2,101	355.15	0.0	0.0	0.0
28.00	0.0	1,870	354.73	0.0	0.0	0.0
30.00	0.0	1,640	354.32	0.0	0.0	0.0
32.00	0.0	1,409	353.92	0.0	0.0	0.0
34.00	0.0	1,179	353.53	0.0	0.0	0.0
36.00	0.0	949	353.15	0.0	0.0	0.0
38.00	0.0	718	352.77	0.0	0.0	0.0
40.00	0.0	488	352.40	0.0	0.0	0.0
42.00	0.0	257	352.03	0.0	0.0	0.0
44.00	0.0	27	351.34	0.0	0.0	0.0
46.00	0.0	0	351.25	0.0	0.0	0.0
48.00	0.0	0	351.25	0.0	0.0	0.0
50.00	0.0	0	351.25	0.0	0.0	0.0
52.00	0.0	0	351.25	0.0	0.0	0.0
54.00	0.0	0	351.25	0.0	0.0	0.0
56.00	0.0	0	351.25	0.0	0.0	0.0
58.00	0.0	0	351.25	0.0	0.0	0.0
60.00	0.0	0	351.25	0.0	0.0	0.0
62.00	0.0	0	351.25	0.0	0.0	0.0
64.00	0.0	0	351.25	0.0	0.0	0.0
66.00	0.0	0	351.25	0.0	0.0	0.0
68.00	0.0	0	351.25	0.0	0.0	0.0
70.00	0.0	0	351.25	0.0	0.0	0.0
72.00	0.0	0	351.25	0.0	0.0	0.0
74.00	0.0	0	351.25	0.0	0.0	0.0
76.00	0.0	0	351.25	0.0	0.0	0.0
78.00	0.0	0	351.25	0.0	0.0	0.0
80.00	0.0	0	351.25	0.0	0.0	0.0
82.00	0.0	0	351.25	0.0	0.0	0.0
84.00	0.0	0	351.25	0.0	0.0	0.0
86.00	0.0	0	351.25	0.0	0.0	0.0
88.00	0.0	0	351.25	0.0	0.0	0.0
90.00	0.0	0	351.25	0.0	0.0	0.0
92.00	0.0	0	351.25	0.0	0.0	0.0
94.00	0.0	0	351.25	0.0	0.0	0.0
96.00	0.0	0	351.25	0.0	0.0	0.0

Drawdown Time < 72 Hours

Hydrograph for Pond INF-201: INF-201

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Outflow (cfs)	Discarded (cfs)	Primary (cfs)
0.00	0.0	0	336.20	0.0	0.0	0.0
2.00	0.0	0	336.20	0.0	0.0	0.0
4.00	0.0	0	336.20	0.0	0.0	0.0
6.00	0.0	0	336.20	0.0	0.0	0.0
8.00	0.0	1	336.20	0.0	0.0	0.0
10.00	0.3	115	336.29	0.2	0.2	0.0
12.00	4.8	4,559	338.25	2.7	0.2	2.5
14.00	1.1	3,798	337.92	1.2	0.2	1.0
16.00	0.6	3,414	337.76	0.6	0.2	0.5
18.00	0.3	3,152	337.66	0.4	0.2	0.2
20.00	0.3	3,006	337.60	0.3	0.2	0.1
22.00	0.2	2,913	337.56	0.2	0.2	0.1
24.00	0.2	2,804	337.52	0.2	0.2	0.0
26.00	0.0	1,755	337.11	0.2	0.2	0.0
28.00	0.0	535	336.61	0.2	0.2	0.0
30.00	0.0	0	336.20	0.0	0.0	0.0
32.00	0.0	0	336.20	0.0	0.0	0.0
34.00	0.0	0	336.20	0.0	0.0	0.0
36.00	0.0	0	336.20	0.0	0.0	0.0
38.00	0.0	0	336.20	0.0	0.0	0.0
40.00	0.0	0	336.20	0.0	0.0	0.0
42.00	0.0	0	336.20	0.0	0.0	0.0
44.00	0.0	0	336.20	0.0	0.0	0.0
46.00	0.0	0	336.20	0.0	0.0	0.0
48.00	0.0	0	336.20	0.0	0.0	0.0
50.00	0.0	0	336.20	0.0	0.0	0.0
52.00	0.0	0	336.20	0.0	0.0	0.0
54.00	0.0	0	336.20	0.0	0.0	0.0
56.00	0.0	0	336.20	0.0	0.0	0.0
58.00	0.0	0	336.20	0.0	0.0	0.0
60.00	0.0	0	336.20	0.0	0.0	0.0
62.00	0.0	0	336.20	0.0	0.0	0.0
64.00	0.0	0	336.20	0.0	0.0	0.0
66.00	0.0	0	336.20	0.0	0.0	0.0
68.00	0.0	0	336.20	0.0	0.0	0.0
70.00	0.0	0	336.20	0.0	0.0	0.0
72.00	0.0	0	336.20	0.0	0.0	0.0
74.00	0.0	0	336.20	0.0	0.0	0.0
76.00	0.0	0	336.20	0.0	0.0	0.0
78.00	0.0	0	336.20	0.0	0.0	0.0
80.00	0.0	0	336.20	0.0	0.0	0.0
82.00	0.0	0	336.20	0.0	0.0	0.0
84.00	0.0	0	336.20	0.0	0.0	0.0
86.00	0.0	0	336.20	0.0	0.0	0.0
88.00	0.0	0	336.20	0.0	0.0	0.0
90.00	0.0	0	336.20	0.0	0.0	0.0
92.00	0.0	0	336.20	0.0	0.0	0.0
94.00	0.0	0	336.20	0.0	0.0	0.0
96.00	0.0	0	336.20	0.0	0.0	0.0

Drawdown Time < 72 Hours

Hydrograph for Pond 1P: Pond

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Primary (cfs)
0.00	0.0	0	320.00	0.0
2.00	0.0	0	320.00	0.0
4.00	0.0	0	320.00	0.0
6.00	0.0	0	320.00	0.0
8.00	0.0	0	320.00	0.0
10.00	0.2	9	320.03	0.2
12.00	6.0	139	320.37	5.8
14.00	2.0	61	320.19	2.0
16.00	1.0	35	320.12	1.0
18.00	0.5	21	320.07	0.5
20.00	0.3	15	320.05	0.3
22.00	0.2	11	320.04	0.2
24.00	0.1	8	320.03	0.1
26.00	0.0	0	320.00	0.0
28.00	0.0	0	320.00	0.0
30.00	0.0	0	320.00	0.0
32.00	0.0	0	320.00	0.0
34.00	0.0	0	320.00	0.0
36.00	0.0	0	320.00	0.0
38.00	0.0	0	320.00	0.0
40.00	0.0	0	320.00	0.0
42.00	0.0	0	320.00	0.0
44.00	0.0	0	320.00	0.0
46.00	0.0	0	320.00	0.0
48.00	0.0	0	320.00	0.0
50.00	0.0	0	320.00	0.0
52.00	0.0	0	320.00	0.0
54.00	0.0	0	320.00	0.0
56.00	0.0	0	320.00	0.0
58.00	0.0	0	320.00	0.0
60.00	0.0	0	320.00	0.0
62.00	0.0	0	320.00	0.0
64.00	0.0	0	320.00	0.0
66.00	0.0	0	320.00	0.0
68.00	0.0	0	320.00	0.0
70.00	0.0	0	320.00	0.0
72.00	0.0	0	320.00	0.0
74.00	0.0	0	320.00	0.0
76.00	0.0	0	320.00	0.0
78.00	0.0	0	320.00	0.0
80.00	0.0	0	320.00	0.0
82.00	0.0	0	320.00	0.0
84.00	0.0	0	320.00	0.0
86.00	0.0	0	320.00	0.0
88.00	0.0	0	320.00	0.0
90.00	0.0	0	320.00	0.0
92.00	0.0	0	320.00	0.0
94.00	0.0	0	320.00	0.0
96.00	0.0	0	320.00	0.0

Drawdown Time < 72 Hours

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location:

TSS Removal Calculation Worksheet

	B BMP ¹	C TSS Removal Rate ¹	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
	Grass Channel	0.50	1.00	0.50	0.50
	Deep Sump and Hooded Catch Basin	0.25	0.50	0.13	0.38
	Extended Dry Detention Basin	0.50	0.38	0.19	0.19
		0.00	0.19	0.00	0.19
		0.00	0.19	0.00	0.19

Total TSS Removal =

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project:
 Prepared By:
 Date:

*Equals remaining load from previous BMP (E) which enters the BMP

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location:

TSS Removal Calculation Worksheet

	B BMP ¹	C TSS Removal Rate ¹	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
	Grass Channel	0.50	1.00	0.50	0.50
	Deep Sump and Hooded Catch Basin	0.25	0.50	0.13	0.38
	Infiltration Basin	0.80	0.38	0.30	0.08
		0.00	0.08	0.00	0.08
		0.00	0.08	0.00	0.08

Total TSS Removal =

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project:
 Prepared By:
 Date:

*Equals remaining load from previous BMP (E) which enters the BMP

Non-automated TSS Calculation Sheet must be used if Proprietary BMP Proposed
 1. From MassDEP Stormwater Handbook Vol. 1

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location:

**TSS Removal
Calculation Worksheet**

B BMP ¹	C TSS Removal Rate ¹	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
Grass Channel	0.50	1.00	0.50	0.50
Deep Sump and Hooded Catch Basin	0.25	0.50	0.13	0.38
Infiltration Basin	0.80	0.38	0.30	0.08
	0.00	0.08	0.00	0.08
	0.00	0.08	0.00	0.08

Total TSS Removal =

**Separate Form Needs to
be Completed for Each
Outlet or BMP Train**

Project:
 Prepared By:
 Date:

*Equals remaining load from previous BMP (E)
which enters the BMP

Non-automated TSS Calculation Sheet
must be used if Proprietary BMP Proposed
1. From MassDEP Stormwater Handbook Vol. 1

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location:

TSS Removal Calculation Worksheet

	B BMP ¹	C TSS Removal Rate ¹	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
	Grass Channel	0.50	1.00	0.50	0.50
	Deep Sump and Hooded Catch Basin	0.25	0.50	0.13	0.38
	Infiltration Basin	0.80	0.38	0.30	0.08
		0.00	0.08	0.00	0.08
		0.00	0.08	0.00	0.08

Total TSS Removal =

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project:
 Prepared By:
 Date:

*Equals remaining load from previous BMP (E) which enters the BMP

Non-automated TSS Calculation Sheet must be used if Proprietary BMP Proposed
 1. From MassDEP Stormwater Handbook Vol. 1

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location:

TSS Removal Calculation Worksheet

	B BMP ¹	C TSS Removal Rate ¹	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
	Grass Channel	0.50	1.00	0.50	0.50
	Deep Sump and Hooded Catch Basin	0.25	0.50	0.13	0.38
	Infiltration Basin	0.80	0.38	0.30	0.08
		0.00	0.08	0.00	0.08
		0.00	0.08	0.00	0.08

Total TSS Removal =

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project:
 Prepared By:
 Date:

*Equals remaining load from previous BMP (E) which enters the BMP

Non-automated TSS Calculation Sheet must be used if Proprietary BMP Proposed
 1. From MassDEP Stormwater Handbook Vol. 1

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location:

TSS Removal Calculation Worksheet

	B BMP ¹	C TSS Removal Rate ¹	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
	Grass Channel	0.50	1.00	0.50	0.50
	Deep Sump and Hooded Catch Basin	0.25	0.50	0.13	0.38
	Infiltration Basin	0.80	0.38	0.30	0.08
		0.00	0.08	0.00	0.08
		0.00	0.08	0.00	0.08

Total TSS Removal =

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project:
 Prepared By:
 Date:

*Equals remaining load from previous BMP (E) which enters the BMP

Non-automated TSS Calculation Sheet must be used if Proprietary BMP Proposed
 1. From MassDEP Stormwater Handbook Vol. 1

Storm Sewer Tabulation

Station		Len (ft)	Drng Area		Rnoff coeff (C)	Area x C		Tc		Rain (l) (in/hr)	Total flow (cfs)	Cap full (cfs)	Vel (ft/s)	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID
Line	To Line		Incr (ac)	Total (ac)		Incr	Total	Inlet (min)	Syst (min)					Size (in)	Slope (%)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	
26	22	10.048	0.11	0.11	0.65	0.07	0.07	5.0	5.0	9.8	0.70	3.85	0.90	12	1.00	319.90	320.00	321.06	321.06	324.44	321.98	Pipe - (01.1)
25	23	14.678	0.48	0.48	0.38	0.18	0.18	7.3	7.3	9.1	1.66	9.09	2.59	18	0.75	320.13	320.24	320.90	320.72	324.67	322.78	Pipe - (03)
24	23	10.751	1.12	1.12	0.42	0.47	0.47	21.9	21.9	6.2	2.94	9.06	3.60	18	0.74	320.13	320.21	320.90	320.86	324.67	322.75	Pipe - (04)
23	22	32.616	0.00	1.60	0.00	0.00	0.65	0.0	22.0	6.2	4.07	8.82	3.61	18	0.71	319.90	320.13	321.06	320.90	324.44	324.67	Pipe - (02)
22	End	48.757	0.00	1.71	0.00	0.00	0.72	0.0	22.3	6.2	4.49	6.73	4.25	18	0.41	319.70	319.90	320.51	320.83	321.35	324.44	Pipe - (01)
21	End	27.248	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	9.70	25.76	5.94	18	6.02	322.00	323.64	324.18	324.84	323.65	325.28	Pipe - (07)
20	End	65.203	1.53	1.53	0.33	0.50	0.50	12.8	12.8	7.7	9.90	27.78	10.42	18	7.00	325.73	330.29	326.35	331.51	327.37	335.76	Pipe - (08)
19	End	27.140	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	3.70	15.08	5.64	18	2.06	326.00	326.56	326.51	327.29	328.65	328.03	Pipe - (05)
18	End	4.278	0.18	0.18	0.61	0.11	0.11	8.3	8.3	8.8	3.97	10.51	4.96	18	1.00	326.56	326.60	327.20	327.36	328.03	335.76	Pipe - (06)
17	End	17.329	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	8.90	19.32	8.36	18	2.89	334.00	334.50	334.72	335.65	335.65	337.18	Pipe - (16)
16	End	19.287	0.11	0.11	0.63	0.07	0.07	5.0	5.0	9.8	0.68	10.42	2.61	18	0.99	335.15	335.34	335.46	335.65	353.65	339.24	Pipe - (17.1)
15	End	62.005	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	5.90	10.50	5.60	18	1.00	346.39	347.01	347.19	347.95	348.04	348.65	Pipe - (09.1)
14	End	6.368	2.11	2.11	0.36	0.76	0.76	14.9	14.9	7.3	5.56	10.19	5.43	18	0.94	347.13	347.19	347.92	348.10	347.27	351.70	Pipe - (10)
13	11	21.705	1.63	1.63	0.30	0.49	0.49	12.4	12.4	7.8	3.82	31.88	8.25	18	9.21	350.00	352.00	350.35	352.75	357.68	359.01	Pipe - (20)
12	11	48.921	0.77	0.77	0.30	0.23	0.23	10.5	10.5	8.2	1.90	10.62	4.02	18	1.02	350.00	350.50	350.43	351.02	357.68	358.00	Pipe - (19)
11	10	101.718	0.00	2.40	0.00	0.00	0.72	0.0	12.6	7.8	5.60	38.68	4.75	18	13.57	335.20	349.00	336.19	349.91	340.50	357.68	Pipe - (18)
10	End	4.750	0.36	2.76	0.40	0.14	0.86	5.0	13.1	7.7	6.63	11.38	6.00	18	1.00	335.15	335.20	335.97	336.19	353.65	340.50	Pipe - (17)
9	8	33.580	0.53	0.53	0.68	0.36	0.36	15.1	15.1	7.3	6.22	3.01	7.92	12	0.71	352.30	352.54	353.30	354.33	358.35	354.29	Pipe - (15)
8	End	3.502	0.00	0.53	0.00	0.00	0.36	0.0	15.2	7.3	6.22	10.50	5.69	18	1.00	352.16	352.20	352.99	353.16	352.26	358.35	Pipe - (14)
7	4	13.372	0.02	0.02	0.70	0.01	0.01	5.0	5.0	9.8	0.14	3.51	1.04	12	0.97	351.19	351.32	351.86	351.47	355.73	353.79	Pipe - (11.4)
6	5	3.502	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	3.00	10.50	1.70	18	1.00	353.62	353.65	356.00	356.00	358.01	355.29	Pipe - (13)
5	4	79.000	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	3.00	24.08	3.96	18	5.26	351.19	355.34	351.86	356.00	355.73	358.01	Pipe - (12)

Common Driveway 85 West Street

Number of lines: 26

Run Date: 4/24/2021

NOTES: Intensity = 127.16 / (Inlet time + 17.80) ^ 0.82; Return period = Yrs. 100 ; c = cir e = ellip b = box

Storm Sewer Tabulation

Station		Len (ft)	Drng Area		Rnoff coeff (C)	Area x C		Tc		Rain (l) (in/hr)	Total flow (cfs)	Cap full (cfs)	Vel (ft/s)	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID
Line	To Line		Incr (ac)	Total (ac)		Incr	Total	Inlet (min)	Syst (min)					Size (in)	Slope (%)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	
4	3	44.215	0.00	0.02	0.00	0.00	0.01	0.0	6.3	9.4	3.13	24.08	4.08	18	5.26	348.86	351.19	349.53	351.86	354.00	355.73	Pipe - (11.3)
3	2	103.853	0.00	0.02	0.00	0.00	0.01	0.0	6.7	9.3	3.13	8.79	4.31	18	0.70	348.13	348.86	348.75	349.53	352.78	354.00	Pipe - (11.2)
2	End	26.772	0.00	0.02	0.00	0.00	0.01	0.0	7.7	9.0	3.13	11.21	4.75	18	0.97	347.77	348.03	348.31	348.70	349.79	352.78	Pipe - (11.1)
1	End	33.022	1.51	1.51	0.36	0.54	0.54	21.7	21.7	6.3	3.41	9.49	4.20	18	0.70	947.13	947.36	947.83	948.06	-0.21	949.80	Pipe - (09.2)

Common Driveway 85 West Street

Number of lines: 26

Run Date: 4/24/2021

NOTES: Intensity = 127.16 / (Inlet time + 17.80) ^ 0.82; Return period = Yrs. 100 ; c = cir e = ellip b = box

NOAA Atlas 14, Volume 10, Version 3
Location name: Westborough, Massachusetts,
USA*



Latitude: 42.2667°, Longitude: -71.6333°
Elevation: 367.67 ft**



* source: ESRI Maps
 ** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps_&_aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour)¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	4.09 (3.11-5.36)	4.86 (3.70-6.40)	6.12 (4.64-8.08)	7.18 (5.40-9.52)	8.63 (6.31-11.9)	9.72 (7.00-13.7)	10.9 (7.61-15.8)	12.1 (8.11-18.1)	13.9 (9.00-21.5)	15.4 (9.73-24.2)
10-min	2.90 (2.20-3.80)	3.44 (2.62-4.53)	4.34 (3.29-5.72)	5.09 (3.83-6.74)	6.11 (4.48-8.44)	6.89 (4.95-9.71)	7.69 (5.39-11.2)	8.59 (5.75-12.8)	9.86 (6.38-15.2)	10.9 (6.89-17.1)
15-min	2.27 (1.73-2.98)	2.70 (2.05-3.55)	3.41 (2.58-4.49)	3.99 (3.00-5.29)	4.79 (3.51-6.62)	5.40 (3.88-7.61)	6.03 (4.23-8.80)	6.74 (4.50-10.1)	7.73 (5.00-11.9)	8.54 (5.41-13.5)
30-min	1.55 (1.17-2.03)	1.84 (1.40-2.42)	2.32 (1.76-3.06)	2.72 (2.05-3.61)	3.27 (2.40-4.52)	3.69 (2.65-5.20)	4.12 (2.89-6.01)	4.60 (3.08-6.87)	5.29 (3.42-8.17)	5.85 (3.70-9.21)
60-min	0.978 (0.743-1.28)	1.17 (0.884-1.53)	1.47 (1.11-1.94)	1.72 (1.30-2.29)	2.07 (1.52-2.86)	2.34 (1.68-3.29)	2.61 (1.83-3.81)	2.92 (1.95-4.36)	3.36 (2.17-5.18)	3.71 (2.35-5.84)
2-hr	0.610 (0.465-0.796)	0.736 (0.562-0.964)	0.944 (0.718-1.24)	1.12 (0.844-1.47)	1.35 (0.998-1.87)	1.53 (1.11-2.16)	1.72 (1.22-2.52)	1.94 (1.30-2.88)	2.27 (1.47-3.49)	2.55 (1.62-3.99)
3-hr	0.464 (0.355-0.605)	0.564 (0.431-0.736)	0.727 (0.554-0.952)	0.862 (0.654-1.14)	1.05 (0.775-1.44)	1.19 (0.863-1.67)	1.34 (0.951-1.96)	1.51 (1.02-2.24)	1.78 (1.16-2.73)	2.01 (1.28-3.13)
6-hr	0.297 (0.228-0.385)	0.361 (0.277-0.469)	0.466 (0.357-0.608)	0.553 (0.421-0.725)	0.674 (0.500-0.924)	0.763 (0.557-1.07)	0.859 (0.614-1.25)	0.975 (0.657-1.44)	1.15 (0.749-1.75)	1.30 (0.829-2.01)
12-hr	0.188 (0.145-0.243)	0.227 (0.175-0.294)	0.292 (0.225-0.379)	0.346 (0.265-0.452)	0.421 (0.313-0.573)	0.476 (0.349-0.663)	0.535 (0.384-0.774)	0.606 (0.410-0.886)	0.712 (0.465-1.08)	0.802 (0.513-1.23)
24-hr	0.113 (0.087-0.145)	0.137 (0.106-0.176)	0.177 (0.136-0.228)	0.210 (0.161-0.272)	0.255 (0.191-0.346)	0.289 (0.213-0.401)	0.325 (0.234-0.469)	0.369 (0.250-0.537)	0.435 (0.285-0.653)	0.491 (0.315-0.750)
2-day	0.063 (0.049-0.081)	0.078 (0.060-0.099)	0.101 (0.078-0.130)	0.121 (0.093-0.156)	0.148 (0.111-0.200)	0.168 (0.124-0.232)	0.190 (0.138-0.273)	0.216 (0.147-0.313)	0.258 (0.169-0.384)	0.293 (0.189-0.445)
3-day	0.046 (0.036-0.058)	0.056 (0.044-0.072)	0.073 (0.057-0.094)	0.087 (0.067-0.112)	0.106 (0.080-0.144)	0.121 (0.090-0.167)	0.136 (0.099-0.196)	0.156 (0.106-0.224)	0.185 (0.122-0.276)	0.211 (0.136-0.319)
4-day	0.037 (0.029-0.047)	0.045 (0.035-0.057)	0.058 (0.045-0.074)	0.069 (0.054-0.089)	0.084 (0.064-0.114)	0.096 (0.071-0.132)	0.108 (0.079-0.154)	0.123 (0.084-0.177)	0.146 (0.096-0.216)	0.166 (0.107-0.250)
7-day	0.025 (0.020-0.032)	0.030 (0.024-0.038)	0.038 (0.030-0.049)	0.045 (0.035-0.058)	0.054 (0.041-0.073)	0.061 (0.046-0.084)	0.069 (0.050-0.097)	0.078 (0.053-0.111)	0.091 (0.060-0.134)	0.103 (0.066-0.154)
10-day	0.020 (0.016-0.026)	0.024 (0.019-0.030)	0.030 (0.023-0.038)	0.035 (0.027-0.045)	0.042 (0.032-0.056)	0.047 (0.035-0.064)	0.052 (0.038-0.073)	0.059 (0.040-0.083)	0.068 (0.045-0.099)	0.075 (0.049-0.113)
20-day	0.014 (0.011-0.018)	0.016 (0.013-0.021)	0.020 (0.015-0.025)	0.022 (0.017-0.028)	0.026 (0.020-0.034)	0.029 (0.021-0.038)	0.031 (0.023-0.043)	0.034 (0.024-0.049)	0.039 (0.026-0.056)	0.042 (0.027-0.062)
30-day	0.012 (0.009-0.015)	0.013 (0.011-0.017)	0.016 (0.012-0.020)	0.017 (0.014-0.022)	0.020 (0.015-0.026)	0.022 (0.016-0.029)	0.024 (0.017-0.032)	0.026 (0.018-0.036)	0.028 (0.019-0.041)	0.030 (0.020-0.044)
45-day	0.010 (0.008-0.012)	0.011 (0.009-0.014)	0.012 (0.010-0.016)	0.014 (0.011-0.017)	0.015 (0.012-0.020)	0.017 (0.013-0.022)	0.018 (0.013-0.025)	0.019 (0.013-0.027)	0.021 (0.014-0.030)	0.022 (0.014-0.032)
60-day	0.009 (0.007-0.011)	0.009 (0.007-0.012)	0.011 (0.008-0.013)	0.012 (0.009-0.015)	0.013 (0.010-0.017)	0.014 (0.010-0.018)	0.015 (0.011-0.020)	0.016 (0.011-0.022)	0.017 (0.011-0.024)	0.018 (0.012-0.026)

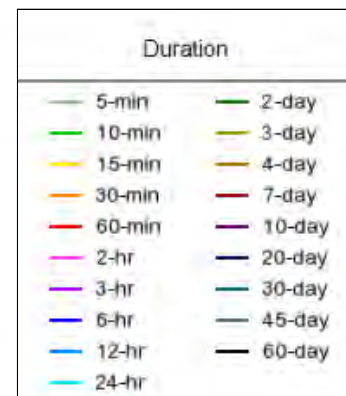
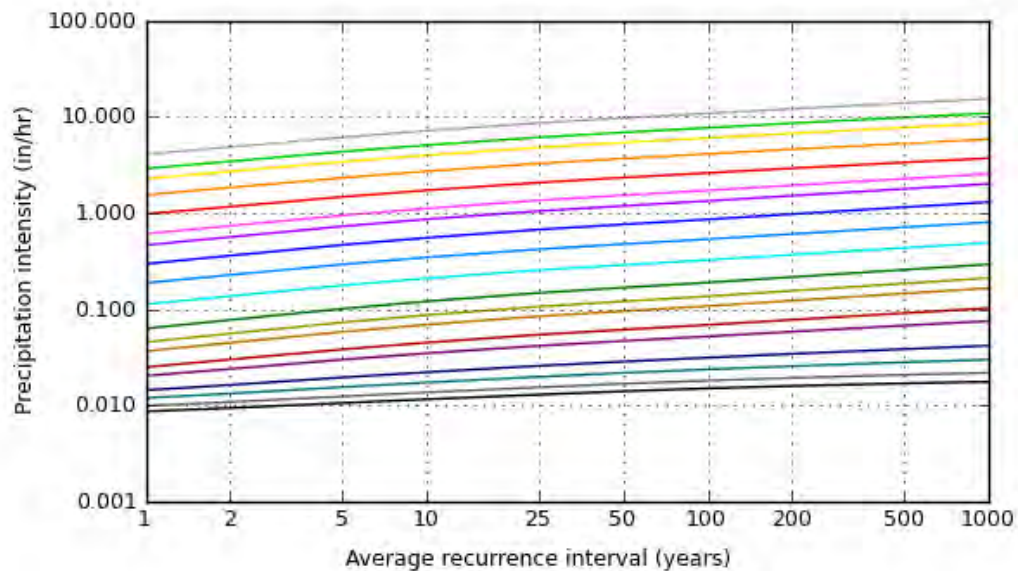
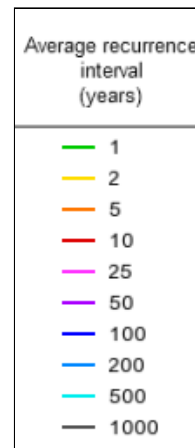
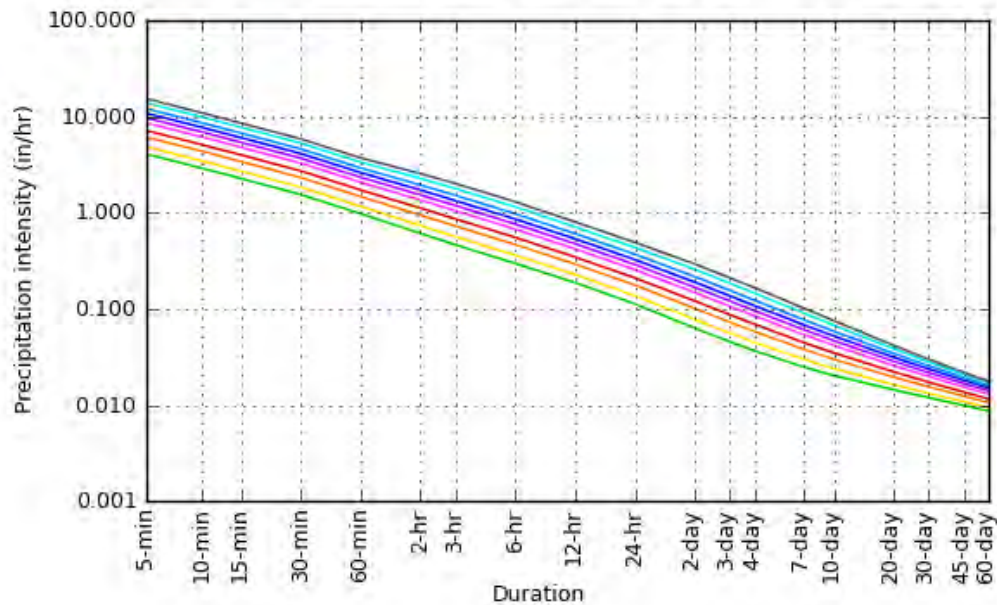
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

[Back to Top](#)

PF graphical

PDS-based intensity-duration-frequency (IDF) curves

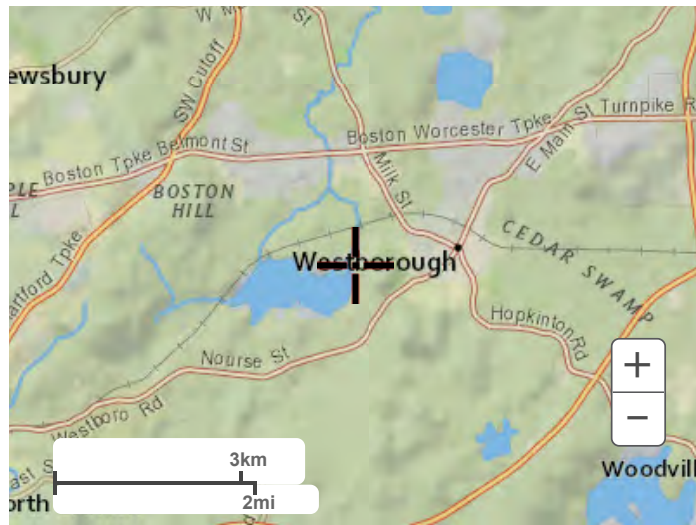
Latitude: 42.2667°, Longitude: -71.6333°



[Back to Top](#)

Maps & aerials

Small scale terrain



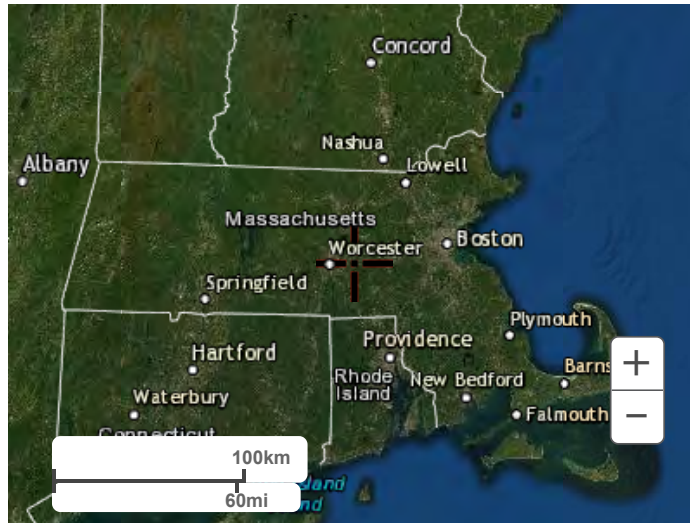
Large scale terrain



Large scale map



Large scale aerial

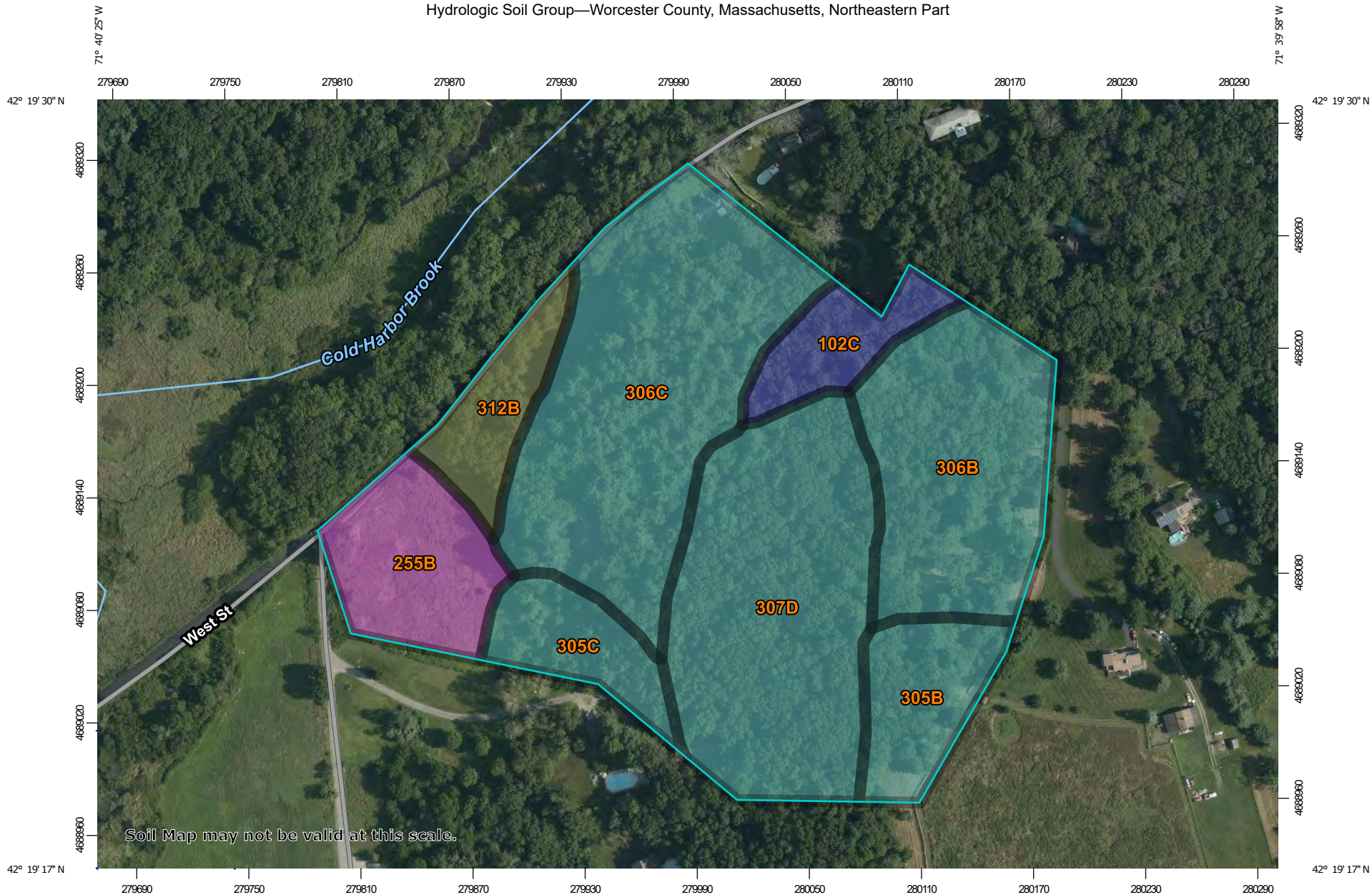


[Back to Top](#)

[US Department of Commerce](#)
[National Oceanic and Atmospheric Administration](#)
[National Weather Service](#)
[National Water Center](#)
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

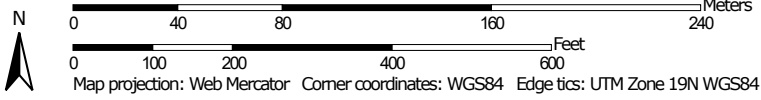
[Disclaimer](#)

Hydrologic Soil Group—Worcester County, Massachusetts, Northeastern Part



































Soil Map may not be valid at this scale.

Map Scale: 1:2,890 if printed on A landscape (11" x 8.5") sheet.



MAP LEGEND

Area of Interest (AOI)		 C	C
 Area of Interest (AOI)		 C/D	C/D
Soils		 D	D
Soil Rating Polygons		 Not rated or not available	Not rated or not available
 A	A	Water Features	
 A/D	A/D	 Streams and Canals	Streams and Canals
 B	B	Transportation	
 B/D	B/D	 Rails	Rails
 C	C	 Interstate Highways	Interstate Highways
 C/D	C/D	 US Routes	US Routes
 D	D	 Major Roads	Major Roads
 Not rated or not available	Not rated or not available	 Local Roads	Local Roads
Soil Rating Lines		Background	
 A	A	 Aerial Photography	Aerial Photography
 A/D	A/D		
 B	B		
 B/D	B/D		
 C	C		
 C/D	C/D		
 D	D		
 Not rated or not available	Not rated or not available		
Soil Rating Points			
 A	A		
 A/D	A/D		
 B	B		
 B/D	B/D		

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Worcester County, Massachusetts, Northeastern Part
 Survey Area Data: Version 15, Jun 10, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 26, 2019—Oct 5, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
102C	Chatfield-Hollis-Rock outcrop complex, 0 to 15 percent slopes	B	1.0	4.9%
255B	Windsor loamy sand, 3 to 8 percent slopes	A	1.8	8.7%
305B	Paxton fine sandy loam, 3 to 8 percent slopes	C	1.4	6.6%
305C	Paxton fine sandy loam, 8 to 15 percent slopes	C	1.1	5.4%
306B	Paxton fine sandy loam, 0 to 8 percent slopes, very stony	C	3.6	17.2%
306C	Paxton fine sandy loam, 8 to 15 percent slopes, very stony	C	5.7	27.5%
307D	Paxton fine sandy loam, 15 to 25 percent slopes, extremely stony	C	5.1	24.3%
312B	Woodbridge fine sandy loam, 0 to 8 percent slopes, extremely stony	C/D	1.1	5.3%
Totals for Area of Interest			20.9	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

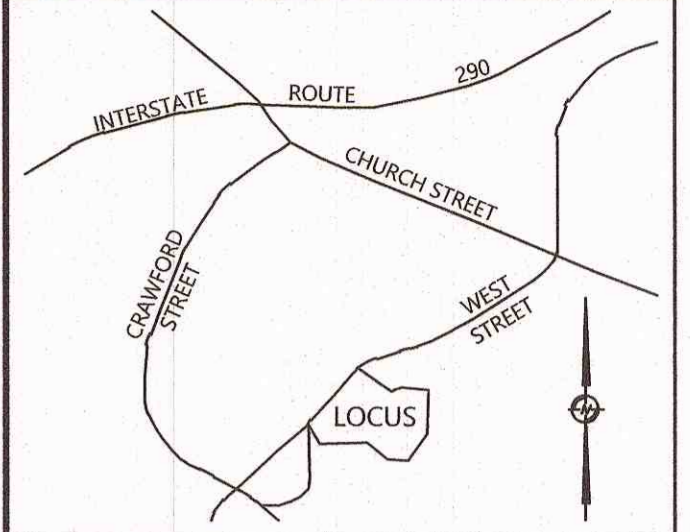
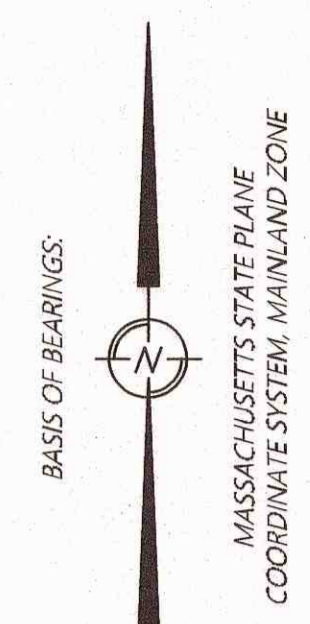
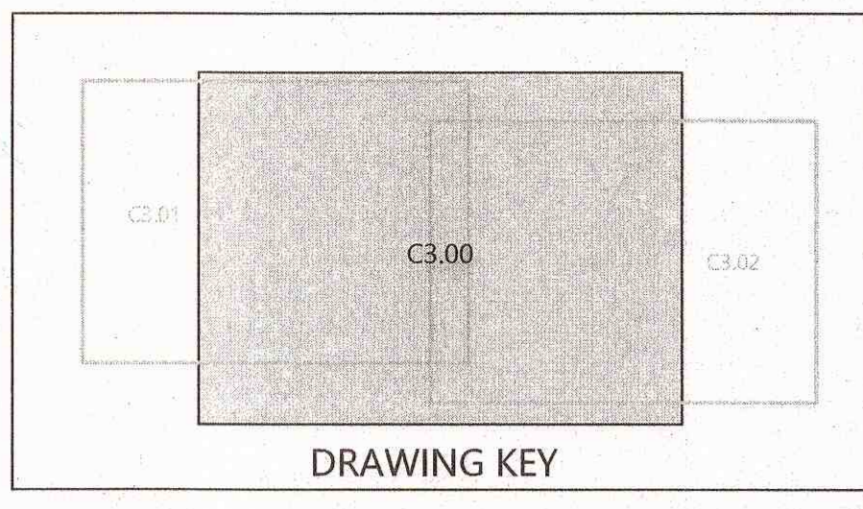
If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

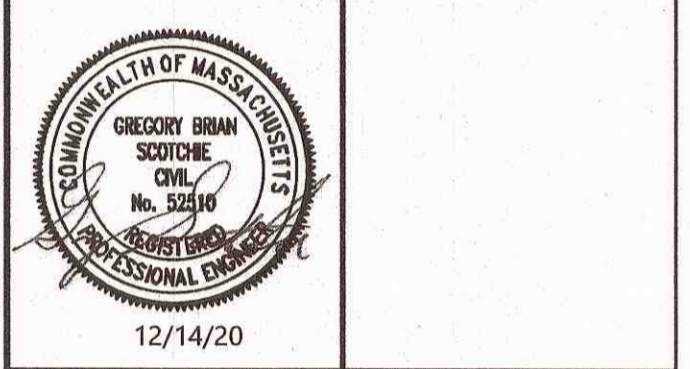
Component Percent Cutoff: None Specified

Tie-break Rule: Higher



THIS DOCUMENT, AS AN INSTRUMENT OF SERVICE, IS THE SOLE PROPERTY OF WDA DESIGN GROUP, INC. ITS USE BY THE OWNER FOR OTHER PROJECTS OR FOR COMPLETION OF THIS PROJECT BY OTHERS IS STRICTLY FORBIDDEN. DISTRIBUTION IN CONNECTION WITH THIS PROJECT SHALL NOT BE CONSTRUED AS PUBLICATION IN VIOLATION OF WDA DESIGN GROUP'S RIGHTS. ©2020 WDA DESIGN GROUP, INC.

REV.	DATE	DESCRIPTION	INIT.
B	12/14/20	CON. COM. COMMENTS	GBS
A		INITIAL ISSUE	GBS



PREPARED BY:
WDA DESIGN GROUP
 31 EAST MAIN STREET, WESTBOROUGH, MA 01581
 508.368.6552
 WDA-DG.COM

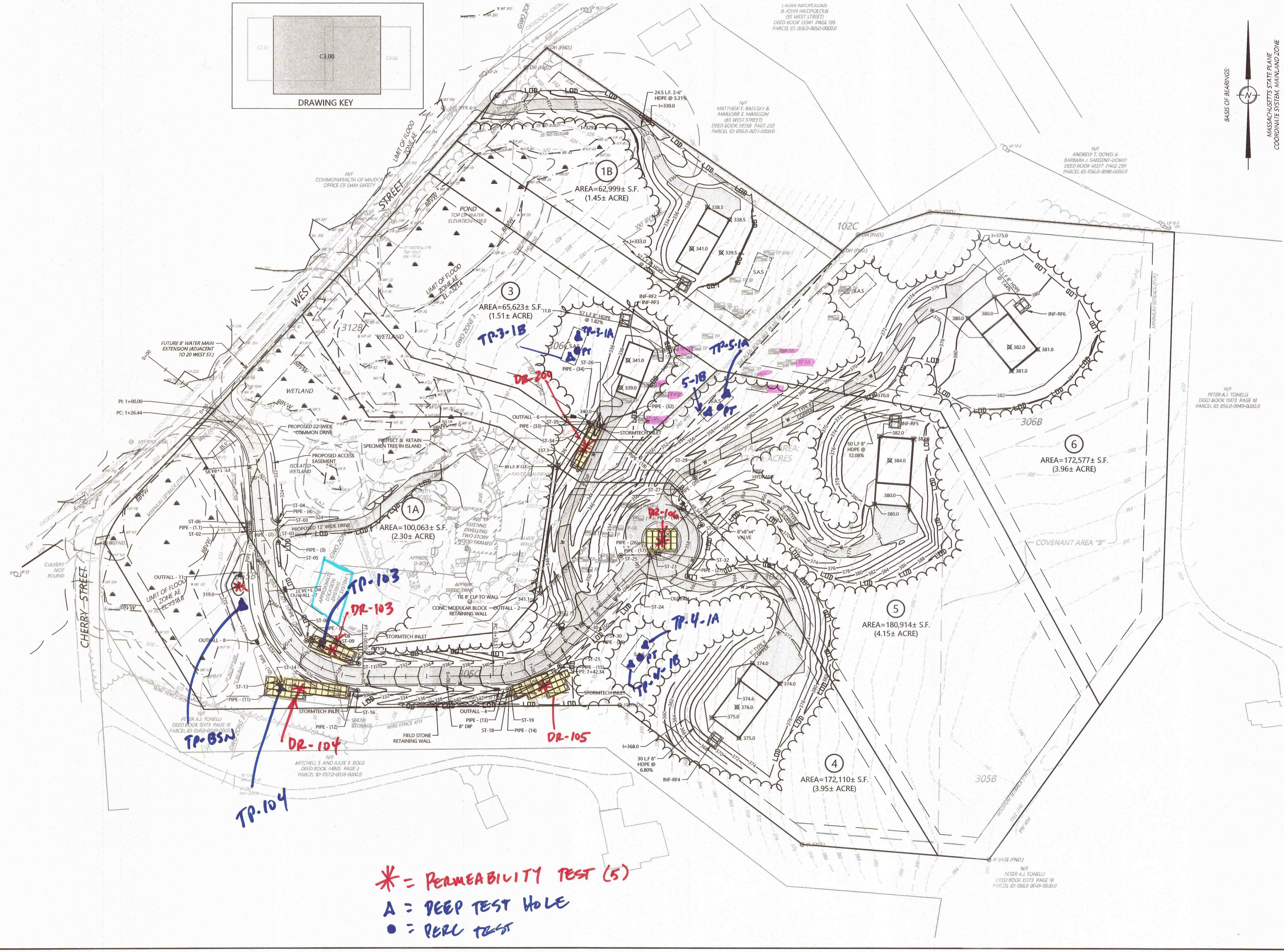
OWNER:
 Brant L. Viner & Margaret Harling
 P.O. Box 295
 Ellsworth, ME 04605

PREPARED FOR:
 Brant L. Viner & Margaret Harling
 P.O. Box 295
 Ellsworth, ME 04605

TITLE:
GRADING, DRAINAGE, & UTILITY PLAN
 85 & 98 COMMON DRIVEWAY
 85 & 95 West Street
 Northborough, MA 01532 (Worcester County)

NOTICE OF INTENT

JOB NO.:	1207.03	DATE:	11/20/20
DWN. BY:	GBS	SHEET:	C3.00
CHK'D. BY:	BPW/JRW		



* = PERMEABILITY TEST (S)
 A = DEEP TEST HOLE
 ● = PERC TEST

Double-Ring Infiltrometer (ASTM D3385-09)



10 Mason Street
Worcester, MA 01609
508-831-7404

Client: WDA Design Group
 Project: 85 West St, Northborough, MA
 Project No.: 2021-1A
 Date: 1/14/2021
 Technician: Mr. Joel Morin
 Weather: Cloudy 32-37F, light snow
 Site Contact: Mr. Neil Stow
 Test No.: DR-103 - see location map

	Area (cm ²)	Depth of Liquid (cm)	Liquid No.	Container vol/ ΔH (cm ³ /mark)	
Test Elevation <u>1.6</u> feet bgs	Inner Ring 726	7.6	1	51.13	Small Mariotte
GW Elevation: <u>n/a</u> feet bgs	Annular Space 2166	7.6	2	161.02	Big Mariotte
Ground Temp: <u>1.7</u> °C					

Trial		Time (min)	Inner Ring		Annular Space		Liquid Temp (°C)	Inner Infiltration Rate (cm/hr)	Annular Infiltration Rate (cm/hr)	Remarks
			Reading	Water Volume (cm ³)	Reading	Water Volume (cm ³)				
1	Test	15	55.00	1150.425	57.50	3639.052	12.2	6.34	6.72	Soil Sample L29511
	Total	15	32.50		34.90					
2	Test	15	32.50	1140.199	34.90	2930.564	12.2	6.28	5.41	
	Total	30	10.20		16.70					
3	Test	15	30.20	1094.182	51.20	2833.952	12.2	6.03	5.23	
	Total	45	8.80		33.60					
4	Test	15	50.70	562.43	51.00	3429.726	12.2	3.10	6.33	
	Total	60	39.70		29.70					
5	Test	15	39.70	485.735	40.00	3542.44	12.2	2.68	6.54	
	Total	75	30.20		18.00					
6	Test	15	55.00	490.848	57.00	2962.768	12.2	2.70	5.47	
	Total	90	45.40		38.60					
7	Test	15	45.40	132.938	49.00	3043.278	12.2	0.73	5.62	
	Total	105	42.80		30.10					
8	Test	15	42.80	127.825	41.00	2978.87	12.2	0.70	5.50	
	Total	120	40.30		22.50					

bgs = below ground surface

Double-Ring Infiltrometer (ASTM D3385-09)



Client: WDA Design Group
 Project: 85 West St, Northborough, MA
 Project No.: 2021-1A
 Date: 1/14/2021
 Technician: Mr. Joel Morin
 Weather: Cloudy 32-37F, light snow
 Site Contact: Mr. Neil Stow
 Test No.: DR-104 - see location map

10 Mason Street
 Worcester, MA 01609
 508-831-7404

	Area (cm ²)	Depth of Liquid (cm)	Liquid No.	Container vol/ Δ H (cm ³ /mark)		
Test Elevation <u>1.5</u> feet bgs	Inner Ring	726	7.6	1	51.13	Small Mariotte
GW Elevation: <u>n/a</u> feet bgs	Annular Space	2166	7.6	2	161.02	Big Mariotte
Ground Temp: <u>1.7</u> °C						

Trial		Time (min)	Inner Ring		Annular Space		Liquid Temp (°C)	Inner Infiltration Rate (cm/hr)	Annular Infiltration Rate (cm/hr)	Remarks
			Reading	Water Volume (cm ³)	Reading	Water Volume (cm ³)				
1	Test	15	57.00	1666.838	58.10	5442.476	12.2	9.18	10.05	Soil Sample L29512
	Total	15	24.40		24.30					
2	Test	15	52.90	1406.075	43.50	5667.904	12.2	7.75	10.47	
	Total	30	25.40		8.30					
3	Test	15	55.10	1124.86	58.10	4267.03	12.2	6.20	7.88	
	Total	45	33.10		31.60					
4	Test	15	56.10	1232.233	50.90	4234.826	12.2	6.79	7.82	
	Total	60	32.00		24.60					

bgs = below ground surface



HYDRAULIC CONDUCTIVITY BY GUELPH PERMEAMETER

Project No.:	2021-1A	Date:	January 20, 2021
Project Name:	85 West Double Rings	Project Address:	85 West Street, Northborough, MA
Client Name:	WDA Design Group	Client Address:	31 East Main Street, Westborough, MA 01581

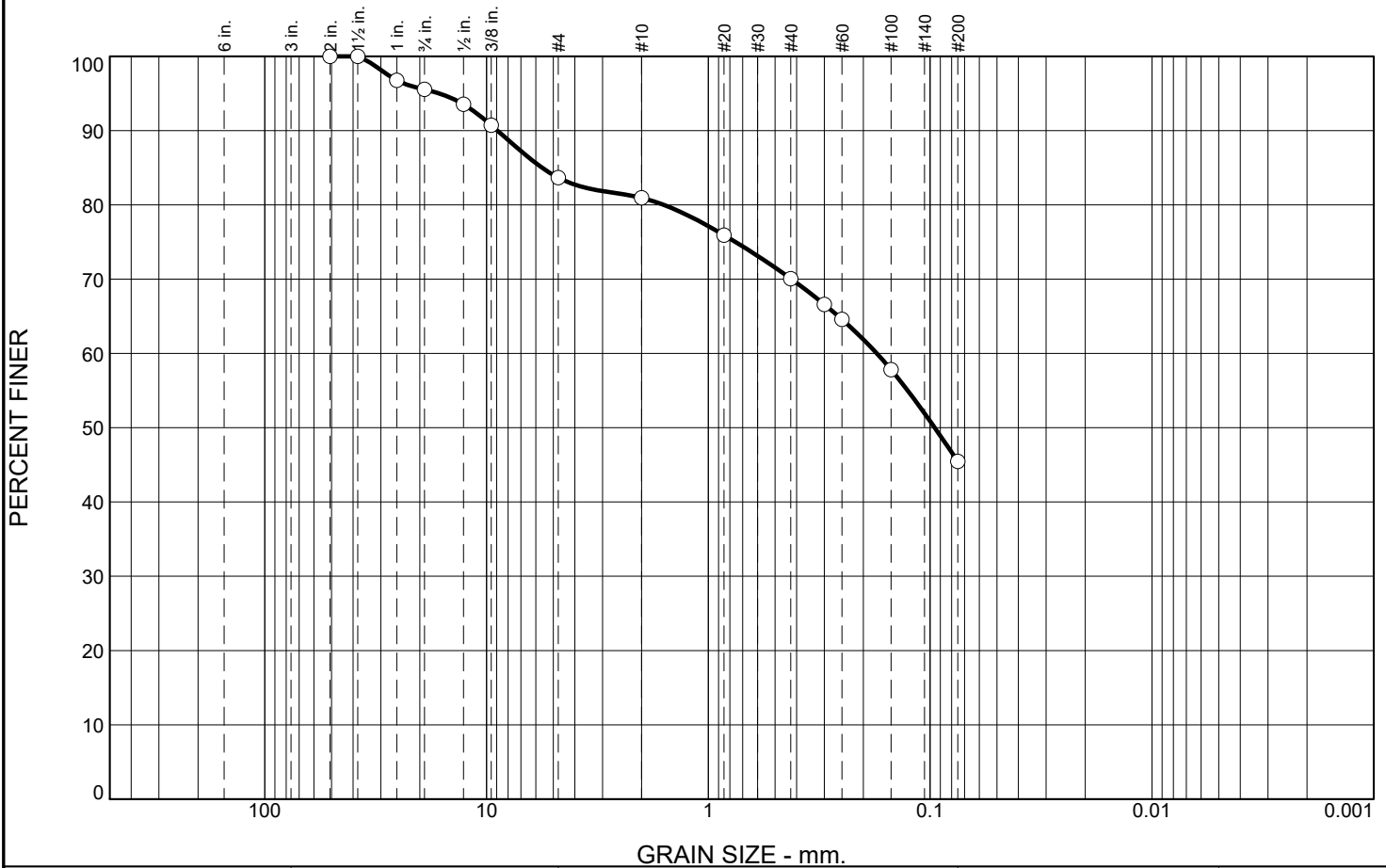
Purpose: To conduct infiltration testing by Guelph Permeameter per ASTM D5126-10

Date Tested	Location	Depth Below Surface Grade (in)	Approx. Test Elevation (ft)	K_{fsat} (in/min)
1/15/21	DR-200 (see location map)	18	336	3.77×10^{-2}
1/15/21	DR-106 (see location map)	18	356	2.93×10^{-2}
1/15/21	DR-105 (see location map)	18	346	2.14×10^{-2}

Data/worksheets available upon request.

Report by: Mr. Joel Morin

Particle Size Distribution Report



% +3"	% Gravel	% Sand	% Silt	% Clay
0.0	16.3	38.2	45.5	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
2"	100.0		
1-1/2"	100.0		
1"	96.8		
3/4"	95.6		
1/2"	93.5		
3/8"	90.7		
#4	83.7		
#10	81.0		
#20	75.9		
#40	70.1		
#50	66.6		
#60	64.6		
#100	57.8		
#200	45.5		

Material Description

Brown 1.5" max sand silt little gravel

Atterberg Limits

PL= NP LL= NV PI= NP

Coefficients

D₈₅= 5.6098 D₆₀= 0.1740 D₅₀= 0.0954
D₃₀= D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= SM AASHTO= A-4(0)

Remarks

Sampled by Joel Morin 1/15/20
See infiltration reports for additional information

* (no specification provided)

Sample No.: L29511
Location: DR-103

Source of Sample: 85 West Double Rings - Northboro

Date: 1/20/21
Elev./Depth: 18" BSG

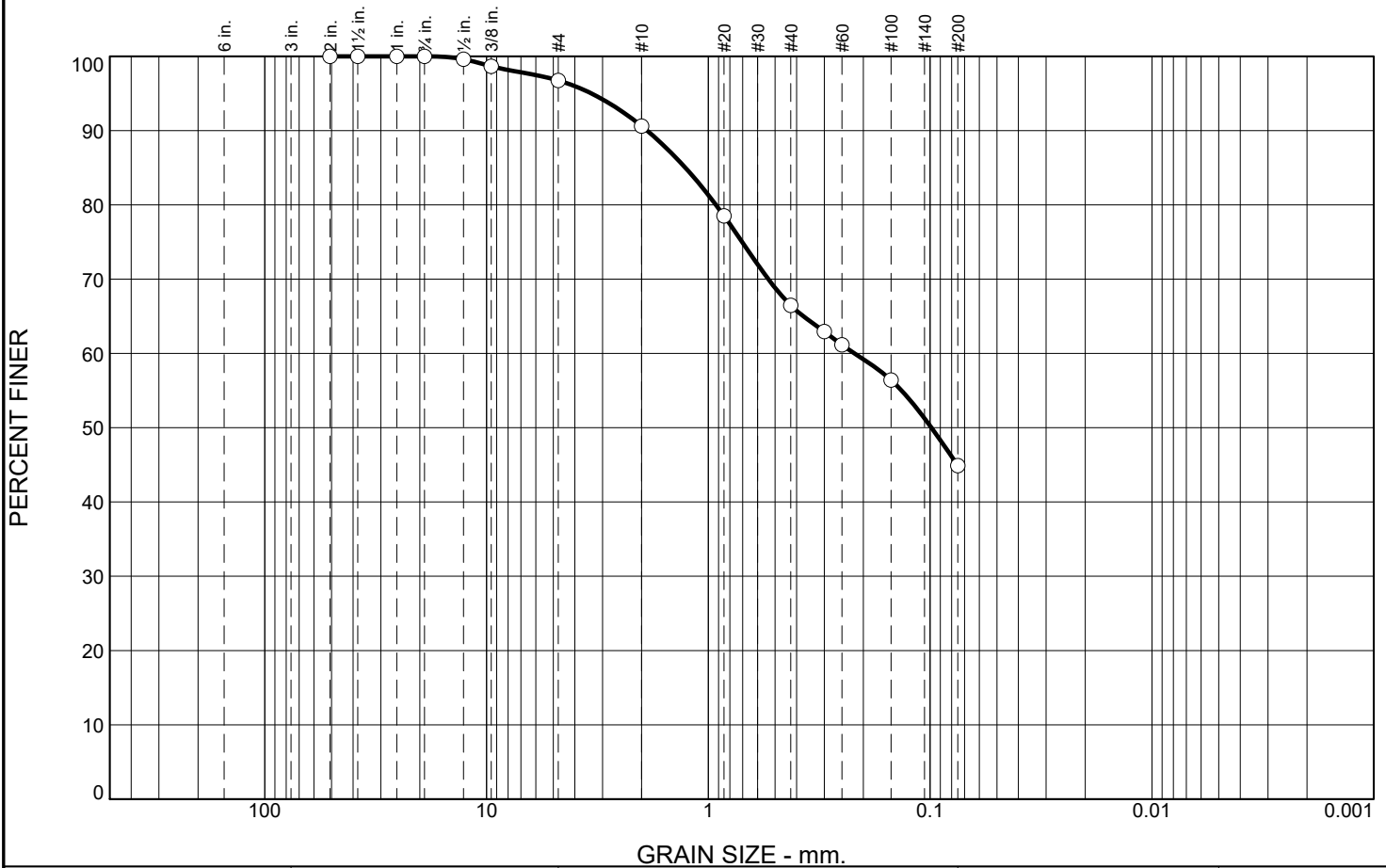
YANKEE ENGINEERING
& TESTING, INC.

Client: WDA Design Group
Project: WDA Design Group
Various Projects/Sites
Project No.: 2020.5

Tested By: AK

Checked By: SMM

Particle Size Distribution Report



% +3"	% Gravel	% Sand	% Silt	% Clay
0.0	3.2	51.9	44.9	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
2"	100.0		
1-1/2"	100.0		
1"	100.0		
3/4"	100.0		
1/2"	99.6		
3/8"	98.7		
#4	96.8		
#10	90.6		
#20	78.5		
#40	66.5		
#50	62.9		
#60	61.2		
#100	56.4		
#200	44.9		

Material Description

Light brown silty sand trace gravel

Atterberg Limits

PL= NP LL= NV PI= NP

Coefficients

D₈₅= 1.2693 D₆₀= 0.2195 D₅₀= 0.0986
D₃₀= D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= SM AASHTO= A-4(0)

Remarks

Sampled by Joel Morin 1/15/20
See infiltration reports for additional information

* (no specification provided)

Sample No.: L29512
Location: DR-104

Source of Sample: 85 West Double Rings - Northboro

Date: 1/20/21
Elev./Depth: 18" BSG

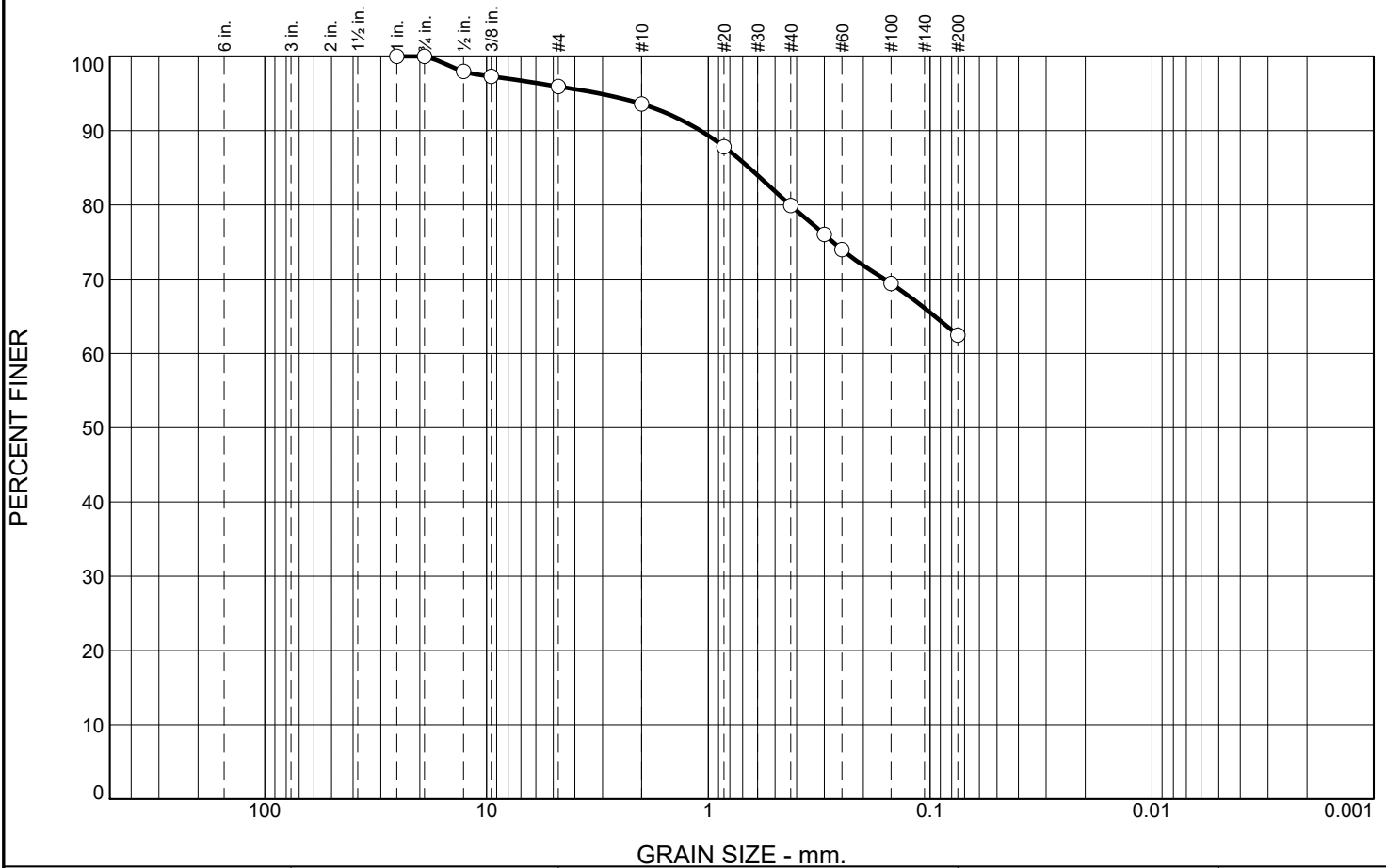
YANKEE ENGINEERING
& TESTING, INC.

Client: WDA Design Group
Project: WDA Design Group
Various Projects/Sites
Project No.: 2020.5

Tested By: AK

Checked By: SMM

Particle Size Distribution Report



% +3"	% Gravel	% Sand	% Silt	% Clay
0.0	4.1	33.4	62.5	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1"	100.0		
3/4"	100.0		
1/2"	98.0		
3/8"	97.3		
#4	95.9		
#10	93.6		
#20	87.8		
#40	79.9		
#50	76.0		
#60	74.0		
#100	69.4		
#200	62.5		

Material Description

Light brown sandy silty trace gravel

Atterberg Limits

PL= NP LL= NV PI= NP

Coefficients

D₈₅= 0.6542 D₆₀= D₅₀=
D₃₀= D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Sampled by Joel Morin 1/15/20
See infiltration reports for additional information

* (no specification provided)

Sample No.: L29513
Location: DR-105

Source of Sample: 85 West Double Rings - Northboro

Date: 1/20/21
Elev./Depth: 18" BSG

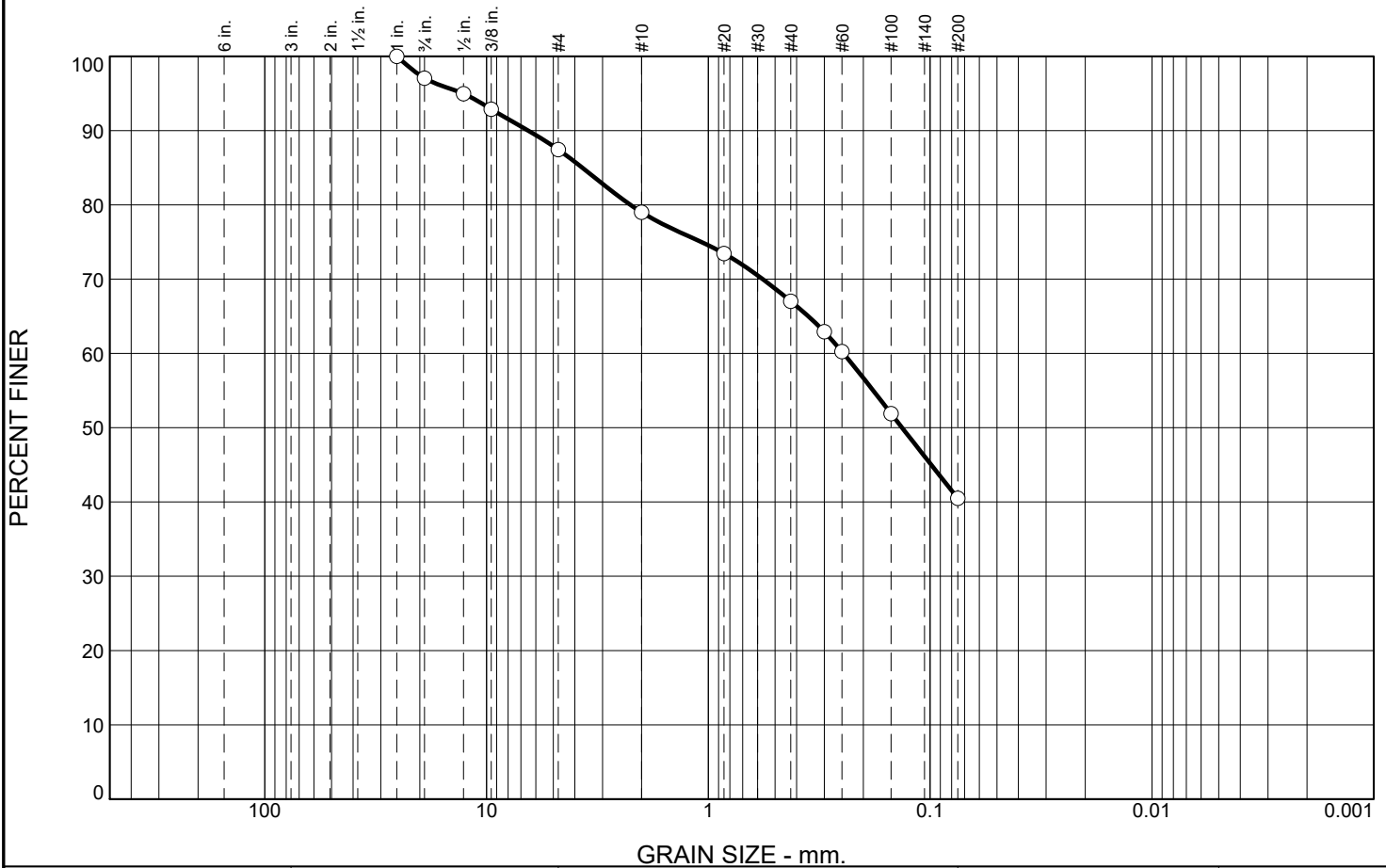
YANKEE ENGINEERING
& TESTING, INC.

Client: WDA Design Group
Project: WDA Design Group
Various Projects/Sites
Project No.: 2020.5

Tested By: AK

Checked By: SMM

Particle Size Distribution Report



% +3"	% Gravel	% Sand	% Silt	% Clay
0.0	12.6	46.9	40.5	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1"	100.0		
3/4"	97.1		
1/2"	95.0		
3/8"	92.9		
#4	87.4		
#10	79.0		
#20	73.5		
#40	67.0		
#50	62.9		
#60	60.3		
#100	51.9		
#200	40.5		

Material Description

Brown silty sand little gravel

Atterberg Limits

PL= NP LL= NV PI= NP

Coefficients

D₈₅= 3.6953 D₆₀= 0.2460 D₅₀= 0.1338
D₃₀= D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= SM AASHTO= A-4(0)

Remarks

Sampled by Joel Morin 1/15/20
See infiltration reports for additional information

* (no specification provided)

Sample No.: L29514
Location: DR-106

Source of Sample: 85 West Double Rings - Northboro

Date: 1/20/21
Elev./Depth: 18" BSG

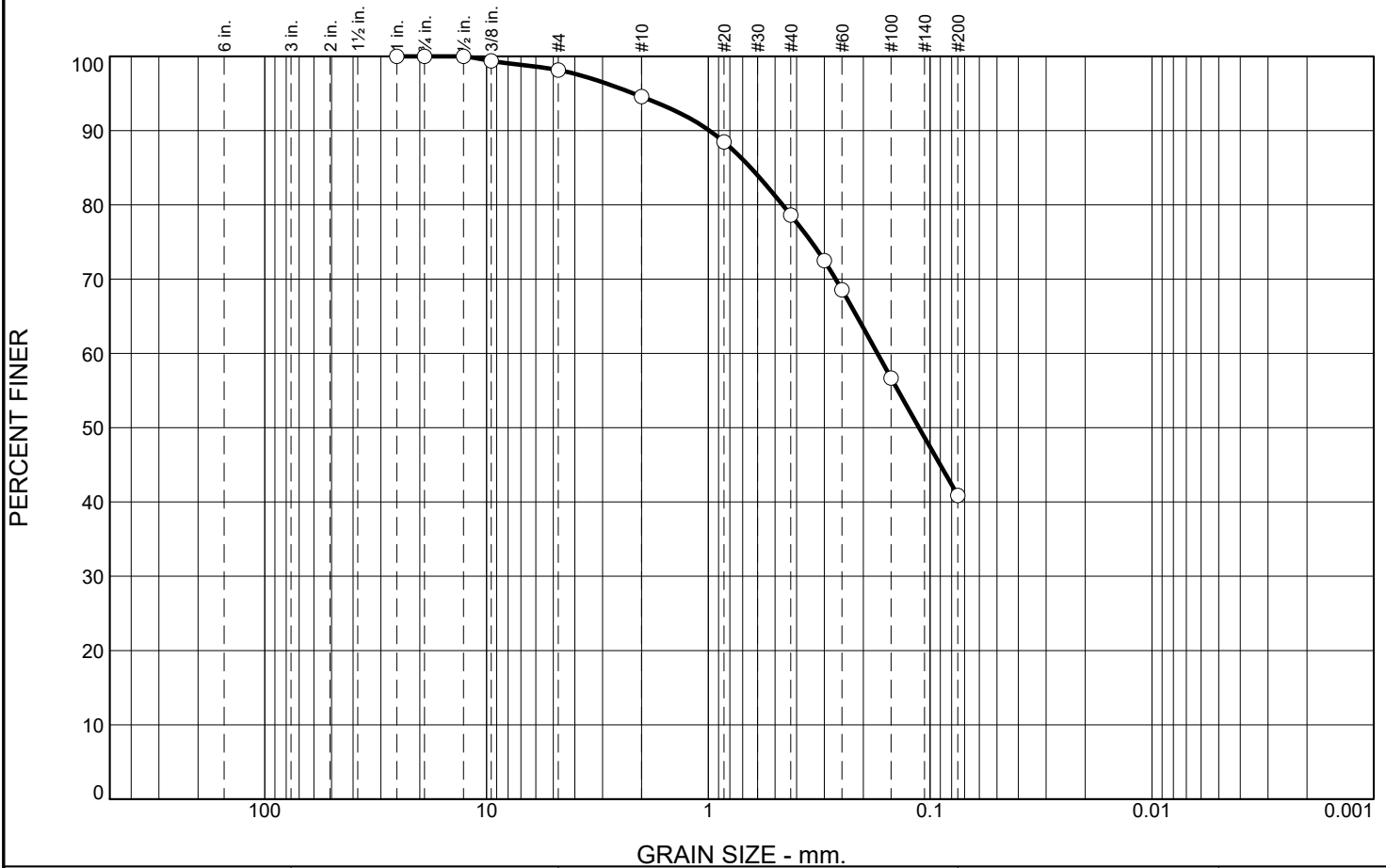
YANKEE ENGINEERING
& TESTING, INC.

Client: WDA Design Group
Project: WDA Design Group
Various Projects/Sites
Project No.: 2020.5

Tested By: AK

Checked By: SMM

Particle Size Distribution Report



% +3"	% Gravel	% Sand	% Silt	% Clay
0.0	1.8	57.3	40.9	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1"	100.0		
3/4"	100.0		
1/2"	100.0		
3/8"	99.4		
#4	98.2		
#10	94.6		
#20	88.5		
#40	78.6		
#50	72.5		
#60	68.6		
#100	56.7		
#200	40.9		

Material Description

Brown silty sand trace gravel

Atterberg Limits

PL= NP LL= NV PI= NP

Coefficients

D₈₅= 0.6441 D₆₀= 0.1728 D₅₀= 0.1122
D₃₀= D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= SM AASHTO= A-4(0)

Remarks

Sampled by Joel Morin 1/15/20
See infiltration reports for additional information

* (no specification provided)

Sample No.: L29516
Location: DR-200

Source of Sample: 85 West Double Rings - Northboro

Date: 1/20/21
Elev./Depth: 18" BSG

**YANKEE ENGINEERING
& TESTING, INC.**

Client: WDA Design Group
Project: WDA Design Group
Various Projects/Sites
Project No.: 2020.5

Tested By: AK

Checked By: SMM

TEST HOLE LOG

85 WEST STREET
NORTHBOROUGH, MA

Date: 1/13/21

Soil Evaluator: Gerald Buzanoski

Excavator: Neil Stowe

TEST HOLE TP-BSN

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-12"	Ap	Fine Sandy Loam	10YR2/2
12"-28"	Bw	Fine Sandy Loam	10YR4/6
28"-87"	C	Loam-FSL	10YR4/4

Comments: Redox features observed at 16", seepage at 20"

TEST HOLE DR-103

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-16"	Ap	Fine Sandy Loam	10YR3/4
16"-38"	Bw	Fine Sandy Loam	10YR4/6
38"-76"	C	Loamy Sand	10YR5/4

Comments: Seepage at 48"

TEST HOLE DR-104

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-16"	Ap	Fine Sandy Loam	10YR3/2
16"-36"	Bw	Fine Sandy Loam	10YR5/6
36"-66"	C	Medium Sand	10YR4/4

Comments: Redox observed at 24", groundwater at 43"

TEST HOLE 3-1A

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-6"	A	Fine Sandy Loam	10YR3/2
6"-50"	Bw	Fine Sandy Loam	10YR4/4
50"-68"	Cd	Loamy Sand	10YR5/3

Comments: Redox observed at 30"; groundwater at 62"

TEST HOLE 3-1B

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-8"	A	Fine Sandy Loam	10YR3/2
8"-24"	Bw	Fine Sandy Loam	10YR4/4
24"-38"	B/C	Loamy Sand	10YR4/4-4/6
38"-103"	Cd	Loamy Sand	10YR5/3

Comments: Redox observed at 32"; seepage at 68"

TEST HOLE 4-1A

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-6"	A	Fine Sandy Loam	10YR3/2
6"-38"	Bw	Fine Sandy Loam	10YR4/4
38"-106"	Cd	Fine Sandy Loam	10YR5/4

Comments: Redox observed at 29"; no groundwater observed

TEST HOLE 4-1B

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-6"	A	Fine Sandy Loam	10YR3/2
6"-46"	Bw	Fine Sandy Loam	10YR4/6
38"-100"	Cd	Fine Sandy Loam	10YR5/2-5/3

Comments: Redox observed at 24"; no groundwater observed

TEST HOLE 5-1A

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-6"	A	Fine Sandy Loam	10YR3/2
6"-52"	Bw	Fine Sandy Loam	10YR4/4
52"-104"	Cd	Fine Sandy Loam	10YR5/3

Comments: Redox observed at 42"; no groundwater observed

TEST HOLE 5-1B

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-6"	A	Fine Sandy Loam	10YR3/2
6"-48"	Bw	Fine Sandy Loam	10YR4/4
48"-96"	Cd	Fine Sandy Loam	10YR5/2-5/3

Comments: Redox observed at 30"; no groundwater observed

TEST HOLE 5-1C

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-6"	A	Fine Sandy Loam	10YR3/2
6"-50"	Bw	Fine Sandy Loam	10YR4/4
50"-88"	Cd	Fine Sandy Loam	10YR5/2-5/3

Comments: Redox observed at 32"; no groundwater observed

PERCOLATION TEST DATA

85 WEST STREET
NORTHBOROUGH, MA

Date: 1/13/21
Soil Evaluator: Gerald Buzanoski
Excavator: Neil Stowe

PERC 3

Depth: 55"
Soak: 1:32
12": 1:47
9": 2:25
6": discontinued
Rate: 40 mpi (estimate)

Comments: 7.5" at 3:03; estimated rate = 40 mpi

PERC 4

Depth: 50"
Soak: 10:46
12": 11:01
9": 11:35
6": 12:30
Rate: 19 mpi

PERC 5

Depth: 50"
Soak: 12:20
12": 12:35
9": 1:08
6": 1:56
Rate: 16 mpi

TEST HOLE LOG

8 West Street
NORTHBOROUGH, MA

Date: 2/3/21

Soil Evaluator: Gerald Buzanoski
Excavator: Neal Stowe, Stowe Excavating

TEST HOLE DR-105

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-6"	A	Fine Sandy Loam	10yr3/3
6"-32"	Bw	Fine Sandy Loam	10YR5/6
32"-50"	Cd	Fine Sandy Loam	10YR5/3

Comments: Redox features observed at 37"; no groundwater observed.

TEST HOLE DR-106

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-6"	A	Fine Sandy Loam	10yr3/3
6"-32"	Bw	Fine Sandy Loam	10YR5/6
32"-80"	Cd	Fine Sandy Loam	10YR5/3

Comments: Faint redox features observed at 70"; no groundwater observed.

TEST HOLE DR-200

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-6"	A	Fine Sandy Loam	10yr3/2
6"-24"	Bw	Fine Sandy Loam	10YR5/6
24"-72"	C1	Loamy Sand	10YR4/6
48"-72"	C2	Silty Fine Sand	10YR4/4

Comments: Distinct redox features observed at 36"; groundwater observed at 60".

85 West Street Test Hole Data (continued)

TEST HOLE TP-5D

<u>Depth</u>	<u>Horizon</u>	<u>Texture</u>	<u>Color</u>
0-6"	A	Fine Sandy Loam	10yr3/2
6"-30"	Bw	Fine Sandy Loam	10YR4/4
30"-85"	Cd	Fine Sandy Loam	10YR5/3

Comments: Few faint redox features observed at 42"; no groundwater observed.

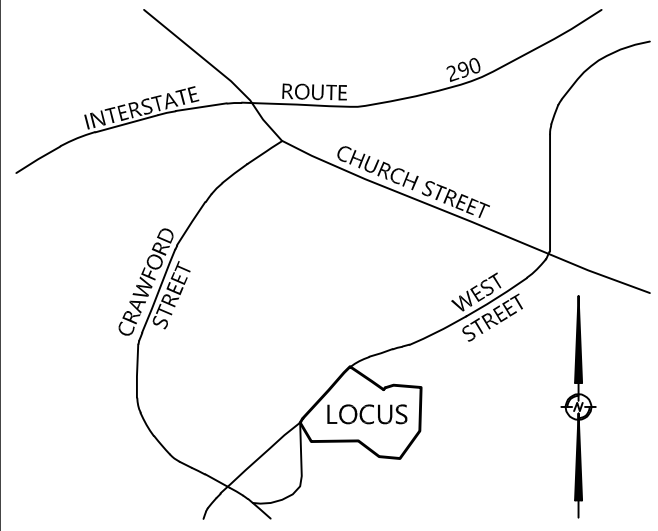


Laura NIKOPOLOUS
& JOHN NIKOPOLOUS
(55 WEST STREET)
DEED BOOK 55941 PAGE 195
PARCEL ID: 056.0-0050-0000.0

N/F
MATTHEW F. BAEVSKY &
MARJORIE E. MARKSON
(65 WEST STREET)
DEED BOOK 39358 PAGE 250
PARCEL ID: 056.0-0051-0000.0

N/F
ANDREW T. DOWD &
BARBARA J. SARGENT-DOWD
DEED BOOK 41377 PAGE 289
PARCEL ID: 056.0-0048-0000.0

MASSACHUSETTS STATE PLANE
COORDINATE SYSTEM, MAINLAND ZONE



LOCUS MAP
(NOT TO SCALE)

THIS DOCUMENT, AS AN INSTRUMENT OF SERVICE, IS THE SOLE PROPERTY OF WDA DESIGN GROUP, INC. ITS USE BY THE OWNER FOR OTHER PROJECTS OR FOR COMPLETION OF THIS PROJECT BY OTHERS IS STRICTLY FORBIDDEN. DISTRIBUTION IN CONNECTION WITH THIS PROJECT SHALL NOT BE CONSTRUED AS PUBLICATION IN DEROGATION OF WDA DESIGN GROUP'S RIGHTS. ©2018, WDA DESIGN GROUP, INC.

REV.	DATE	DESCRIPTION	INIT.
G	5/3/2021	TOWN ENGINEER COMMENT	GBS
F	4/26/2021	PLANNING BOARD COMMENTS	GBS
E	4/14/2021	FOR DATE ONLY	GBS
D	3/16/2021	FOR DATE ONLY	GBS
C	2/24/2021	FOR DATE ONLY	GBS
B	12/14/20	FOR DATE ONLY	GBS
A		INITIAL ISSUE	

N/F
PETER A.J. TONELLI
DEED BOOK 15173 PAGE 18
PARCEL ID: 056.0-0049-0000.0

TOTAL LOT AREA:
17.31± ACRES

PREPARED BY:



31 EAST MAIN STREET WESTBOROUGH, MA | 508.366.6552
7 CENTRAL STREET PROVIDENCE, RI | 401.274.1360
WDA-DG.COM

OWNER:

Brant L. Viner & Margaret
Harling
P.O. Box 295
Ellsworth, ME 04605

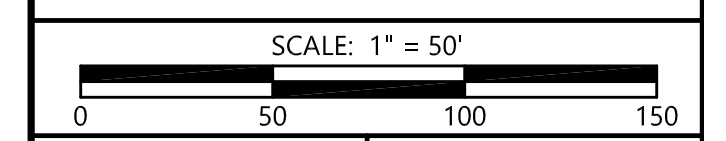
PREPARED FOR:

Brant L. Viner & Margaret
Harling
P.O. Box 295
Ellsworth, ME 04605

TITLE:

EXISTING
HYDROLOGY
85 & 98 COMMON
DRIVEWAY
85 & 95 West Street
Northborough, MA 01532
(Worcester County)

NOTICE OF INTENT



JOB NO.:	1207.03	DATE:	11/20/20
DWN. BY:	GBS	SHEET:	
CHK'D. BY:	BPW/JRW		EX-1

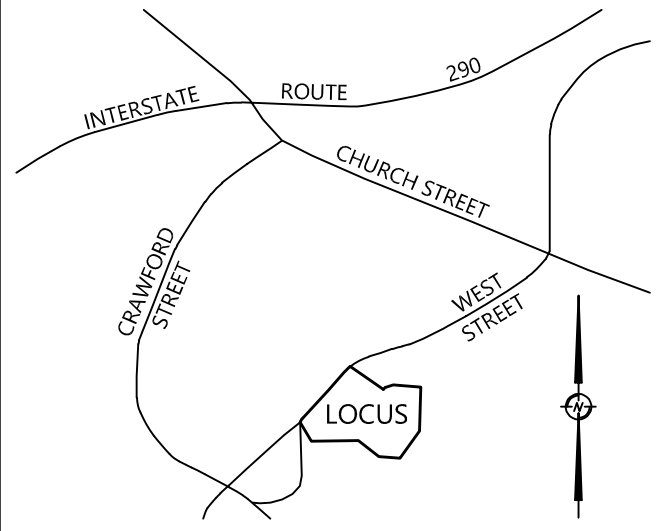


Laura Nikopolous & John Nikopolous
 65 WEST STREET
 DEED BOOK 55941 PAGE 195
 PARCEL ID: 056.0-0050-0000.0

N/F
 MATTHEW F. BAEVSKY &
 MARJORIE E. MARKSON
 105 WEST STREET
 DEED BOOK 39358 PAGE 250
 PARCEL ID: 056.0-0051-0000.0

N/F
 ANDREW T. DOWD &
 BARBARA J. SARGENT-DOWD
 DEED BOOK 41377 PAGE 289
 PARCEL ID: 056.0-0048-0000.0

MASSACHUSETTS STATE PLANE
 COORDINATE SYSTEM, MAINLAND ZONE



LOCUS MAP
 (NOT TO SCALE)

THIS DOCUMENT, AS AN INSTRUMENT OF SERVICE, IS THE SOLE PROPERTY OF WDA DESIGN GROUP, INC. ITS USE BY THE OWNER FOR OTHER PROJECTS OR FOR COMPLETION OF THIS PROJECT BY OTHERS IS STRICTLY FORBIDDEN. DISTRIBUTION IN CONNECTION WITH THIS PROJECT SHALL NOT BE CONSTRUED AS PUBLICATION IN DROGATION OF WDA DESIGN GROUP'S RIGHTS. ©2018, WDA DESIGN GROUP, INC.

REV.	DATE	DESCRIPTION	INIT.
G	5/3/2021	FOR DATE ONLY	GBS
F	4/24/2021	PLANNING BOARD COMMENTS	GBS
E	4/14/2021	FOR DATE ONLY	GBS
D	3/16/2021	FOR DATE ONLY	GBS
C	2/24/2021	PLANNING BOARD COMMENTS	GBS
B	12/14/20	FOR DATE ONLY	GBS
A		INITIAL ISSUE	

PREPARED BY:

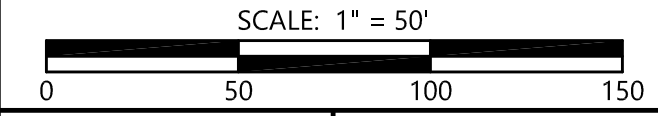
31 EAST MAIN STREET WESTBOROUGH, MA | 508.366.6552
 7 CENTRAL STREET PROVIDENCE, RI | 401.274.1360
 WDA-DG.COM

OWNER:
 Brant L. Viner & Margaret
 Harling
 P.O. Box 295
 Ellsworth, ME 04605

PREPARED FOR:
 Brant L. Viner & Margaret
 Harling
 P.O. Box 295
 Ellsworth, ME 04605

TITLE:
**PROPOSED
 HYDROLOGY**
 85 & 98 COMMON
 DRIVEWAY
 85 & 95 West Street
 Northborough, MA 01532
 (Worcester County)

NOTICE OF INTENT



JOB NO.:	1207.03	DATE:	11/20/20
DWN. BY:	GBS	SHEET:	
CHK'D. BY:	BPW/JRW		PR-1